## EXPERIMENTAL USE OF RESIN MODIFIED ASPHALT PAVEMENT ON MONTPELIER STATE HIGHWAY

INITIAL REPORT 83-5

JUNE 1983

REPORTING ON WORK PLAN 82-R-19

STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH DIVISION

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#### ABSTRACT

Approximately 365 tons of bituminous concrete mix modified with Solar Laglugel, a natural and synthetic resin, was produced and placed as a wearing course on Montpelier State Highway in Montpelier, Vermont in October of 1982.

The resins were added to the asphalt in the plant storage tank at a rate of 1.34 percent by weight of the asphalt cement. No significant problems were encountered with the production or placement of the modified mix. Lab tests conducted on the core samples for recovered penetration values averaged 55 for the standard mix and 61 for the modified mix.

Mays Ride Meter readings taken on the project averaged 49.8"/mile on the regular mix with readings of 36.4"/mile on the modified mix.

Field inspections through the first six (6) months of service revealed less than 1 percent cracking in both the standard and modified mixes. Rutting readings ranged from 1/16" to 3/16" in the standard mix and 1/16" to 2/16" in the modified mix. Both mixes averaged 1/16" overall

#### INTRODUCTION

Solar Laglugel (S.L.) is a resin and nylon base modifier designed to increase the performance of bituminous concrete paving surfaces. Suppliers propose that the use of S.L. with its greater resiliency allows for the placement of thinner overlays resulting in significant cost savings.

In August, 1982, the Vermont Agency of Transportation was offered 550 pounds of the S.L. at no charge for a field evaluation. With the cooperation of the local bituminous concrete producer, Cooley Asphalt Paving Corporation, an experimental bituminous concrete mix was batched and placed in October of 1982.

This report describes the production and placement of the modified mix and field performance results through the first six months of service.

#### PRODUCT INFORMATION PROVIDED BY SUPPLIER

Solar Laglugel is a nylon based modifier consisting of fourteen (14) natural and synthetic resins recommended for use in bituminous concrete mixes. The material donated for experimental use on Vermont highways was Solar Laglugel, distributed by Additives of New England, Inc., 104 Woodland Road, Ashland, Massachusetts, 01721 (phone, 617-881-1684).

The product data indicates that S.L. treated material has a higher Tensil Splitting Strength than untreated mix. Stripping resistance tests of a S.L. sample treated for 24 hours at 60°C showed that 50% of the asphalt was retained. After boiling for 10 minutes in water, both treated and untreated samples retained less than 5% asphalt (these strip test results do not meet State of Vermont Standard Specifications for Highway and Bridge Construction).

The S.L. modifier can be added to the asphalt cement in the storage tank at the batch plant or to the transport tanker. Mixing is achieved by circulating the mixture for 60 minutes at temperatures between  $280^{\circ}$ F and  $320^{\circ}$ F.

Placement and compaction of the modified mix is achieved with conventional pavers and rollers.

When S.L. is added at a rate of 1.33 percent by weight of the asphalt cement, the modified mix could be expected to cost \$10.00/ton extra (1.68 lbs/ton). The material is sold in 110 pound bags for \$5.60/lb., F.O.B. Additives of New England, Ashland, Massachusetts for 1982/1983.

Price includes labor for adding the S.L. to the mix. The supplier claims that adding the modifier to the mix is cost effective because the performance is sufficient to extend pavement life and allow for a significant reduction in pavement thickness (3/4" min., 1" overlay is usually sufficient).

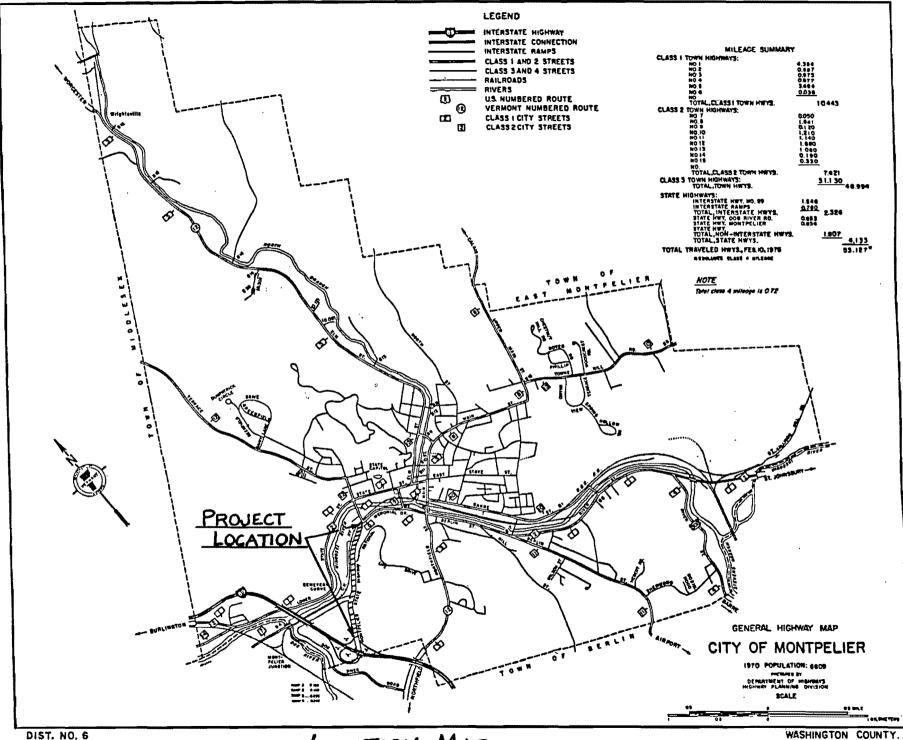
#### PROJECT DESCRIPTION & ROADWAY CONDITION

The project, Montpelier F 028-2(6)S, was located in the City of Montpelier, on the Montpelier State Highway, beginning at the intersection of the Montpelier State Highway and Ramp A of Interstate 89 and extending easterly 3981 feet, ending 200 feet west of the intersection with Bailey Avenue Extension. See location map on page 5.

The existing roadway was constructed during the period 1958-1960 using a 20" Gravel and 6" Macadam Sub-base, with 3" of Bituminous Concrete Pavement. An additional 3/4" of bituminous concrete mix was placed in 1969.

From the centerline of the intersection at Bailey Avenue Extension proceeding westerly, the roadway is undivided for 1096' beginning at that point, there is a median varying in width from 4' - 16' dividing the highway (with the exception of the National Life drive and Montpelier Junction turn) to its termination at the I89 interchange.

Detailed pavement condition surveys were made at six locations approximately 1 month prior to cold planing the roadway. The test section surveys included a detailed crack count and rutting measurments at 3 locations per test



**U** 

LOCATION MAP

section. The crack counts revealed an average of 366 lf. of cracks per 100 lf. of 24' wide roadway. Approximately 59% of the cracks were longitudinal, 36% were transverse, and the remaining 5% were miscellaneous. Rutting ranged from 1/16" to 12/16" with an overall average of 5/16". The pavement's original condition can be seen on pavement survey sheets, Appendix A1 - 8.

The entire length of the project was cold planed at a depth of 1" and the test sections were resurveyed prior to paving. The crack counts showed that 84% of the cracks were still visible. There were 2092 lf. of cracks in the surface prior to cold planing compared to 1747 lf. after cold planing.

The average daily traffic for this section of Montpelier State Highway in 1980 was 7965.

#### MIX PRODUCTION AND TESTING

The 550 pounds of S.L. arrived at the Cooley Asphalt Paving Corp. in Berlin, Vermont on October 7, 1982. The modifier was added to 41,000 pounds of asphalt in storage tank #3 at a rate of 1.34% by weight of the asphalt cement. While adding the modifier to the tank, the workers noted that several of the S.L. bags had arrived in a semisolid state. The supplier indicated this was due to warm storage for several months. To remedy the problem, the plant employees found it necessary to use a steel bar to break up the bags of caked material. The remaining bags crumbled with handling and were easily poured into

the tank. Once all the material was in the tank, the asphalt was circulated overnight at a temperature of  $300^{\circ}$  to  $320^{\circ}$ .

Due to rain, paving was cancelled for October 8, 1982. When visually inspected through the top port of the asphalt tank on October 9, 1982, it appeared that the S.L. modifier was floating on the surface of the asphalt cement. A sampling device was designed so the asphalt could be sampled directly from the top of the tank. Several samples were taken but there was no visual evidence of any undissolved S.L. modifier floating on the surface of the asphalt cement sample.

A Type III bituminous concrete mix was produced for the project on October 11, 1982, at the Cooley Asphalt Paving Corporation batch plant. The asphalt was an 85/100 penetration grade supplied by British Petroleum, Montreal, Quebec, Canada. The coarse aggregate consisted of crushed granite. The fine aggregate included natural sand and stone screenings from crushed granite.

The modified and standard mixes were tested on October 11, 1982, for asphalt content, gradation, air void content, stability and flow.

The following chart shows the results of those tests:

STANDARD MIX

12010 110		STATISTICS TIEM
Extraction (Asphalt content)	6.22%	6.27%
Air Void Content	3.7%	3.9%
Stablilty	2424 <b>1</b> b.	2525 lb.
Marshall Flow Value	10	12

MODIFIED MIX

TESTS

Details of the mix design and laboratory test results can be seen in Appendix C1 - 11.

#### PAVING OPERATION

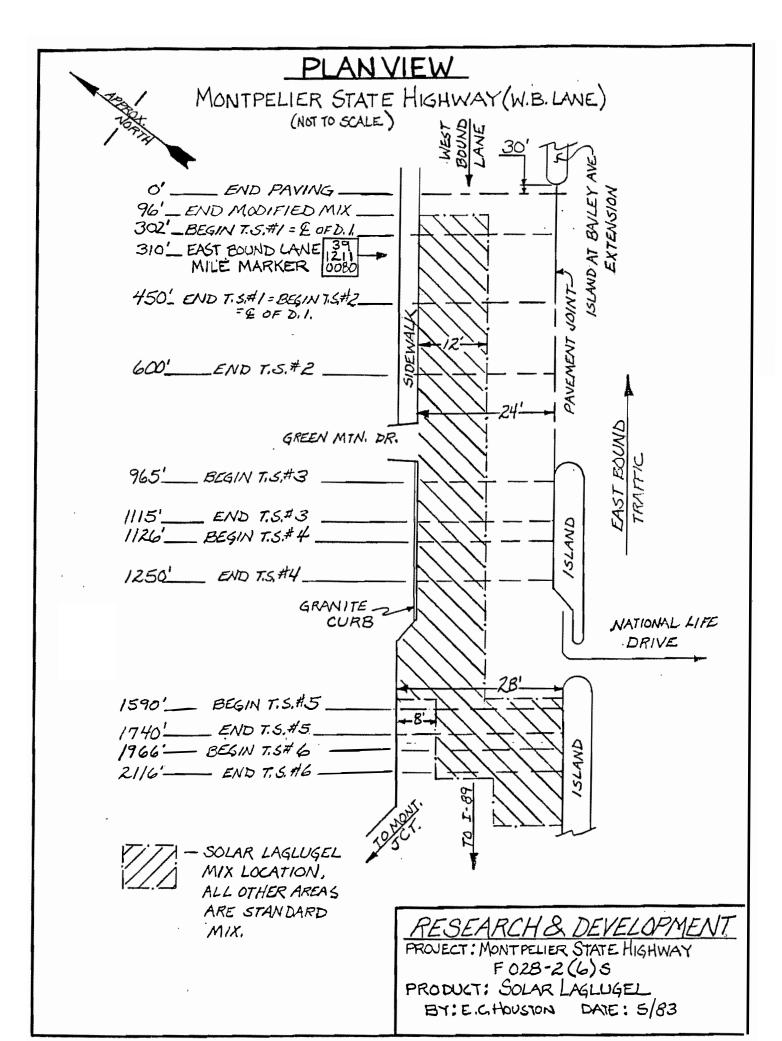
Paving began about 10:00 AM on October 11, 1982 under overcast skies with the ambient temperature at 40°F and rising. Approximately 228 tons of standard mix was produced prior to switching over to the modified mix. A plan view of the installation can be seen on page 9.

The paving sequence proceeded from west to east in the west bound lane.

Type III mix was used until the paver neared the test sections, at which time the tanks at Cooley's were switched to the modified asphalt. The modified mix covered half of all the test sections.

Thickness of the overlay was 1 1/4" throughout the project. Mix temperatures sampled from trucks on the project ranged from 295° to 305°F. Workers on the project commented that the modified mix was stickier than the standard mix but did not present any problems with placement. It appeared somewhat blacker and had more of a shine but the color variation was not evident after the pavement had been compacted. The S.L. modifier mix gave off a very strong odor.

Modified mix was placed in test sections 1-4 between 3:00 PM and 3:25 PM. The breakdown rolling phase left edge and stopping marks in the pavement surface in these areas. Traffic was placed on the roadway in these areas prior to final rolling. The final rolling took place after 5:00 PM when the surface temperature was less than  $90^{\circ}$ F.



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#### POST CONSTRUCTION OBSERVATIONS

Core samples taken November 22, 1982 showed that compaction was within job specifications when tested in the lab. The test results averaged 97.7 for percent compaction (see Core Analysis - Column I, Appendix C-10). Percent air voids ran on the high side with an average of 6.3 (the standard ranges from 2 - 8%).

The cores were also tested for gradation, asphalt content, and recovered penetration of the asphalt cement. Both mix types passed the job requirements for gradation and percent asphalt cement. Penetration values for the recovered asphalt from the cores taken in the modified mix were 63 and 58 for an average of 60.5. The standard mix had somewhat lower values of 58 and 51 with an average of 54.5.

On November 30, 1982, a May Ride Meter Surface Tolerance Run was made over the project area. The weather was cloudy with light rain and a temperature of  $45^{\circ}$ F. Vehicle speed was  $45\pm$  mph.

Readings were as follows:

Type Mix	<u>Lane</u>	Initial Readings (Inches/Mile)
Standard	West Bound,Left	49.8"
Modified	West Bound, Right	36.4"

Test sections 1-4 were surveyed for cracks and rutting on March 23, 1983. (Test sections 5 and 6 were not resurveyed due to the placement of both standard and modified mix within both test sections). Rutting ranged from 1/16" to 3/16". The following chart shows a comparison of crack count data taken after cold planing and after the first six months of service.

#### CRACK SUMMARY CHART\*

Marin (Christian Christian		truction d Planing	Post Cons	truction
	Standard Mix	Modified Mix	Standard Mix	Modified Mix
Total LF.	594	652	2	5
Avg/100'Rdwy.	104	113	0	1

\*Total lineal footage of cracks does not include cracks along the longitudinal pavement joint or test sections 5 and 6.

There were some areas of small transverse parallel cracks located primarily in the area of test sections 1-4. These areas corresponded with the areas which had traffic on them prior to final rolling (as noted on page 8). These finely cracked areas appear to have the potential of developing into larger cracks which allow the intrusion of water. One such area had already developed in test section #4. A crack of about 10" in length appears to be wider than others and has water accumulated in it.

#### SUMMARY

No significant problems were encountered in the production or placement of approximately 365 tons of bituminous concrete mix modified with SOLAR LAGLUGEL.

With the exception of mixing the additive with the asphalt in the storage tank, the mix procedure is basically the same. The bituminous mix producer felt that the mixing process would not significantly alter the normal plant procedures.

Lab tests conducted on the core samples for recovered penetration of the asphalt cement averaged 55 for the standard mix and 61 for the modified mix.

Mays Ride Meter readings taken on the project averaged 49.8"/mile on the regular mix with readings of 36.4"/mile on the modified mix.

After six months of service, crack reflection totalled less than 1% on both the standard and modified mixes. Rutting readings taken prior to cold planing ranged from 1/16" to 12/16". Readings taken after 6 months of service revealed readings ranging from 1/16" to 3/16" in the standard mix and 1/16" to 2/16" in the modified mix. Both mixes averaged 1/16" overall.

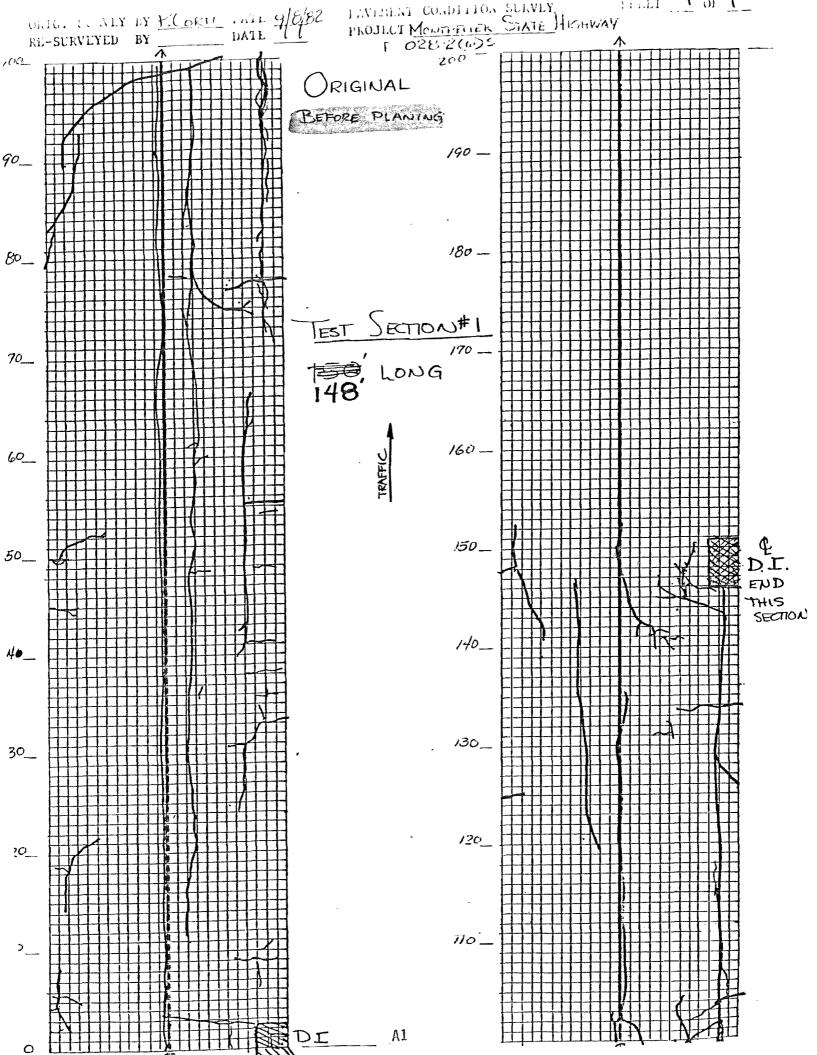
#### FOLLOW UP

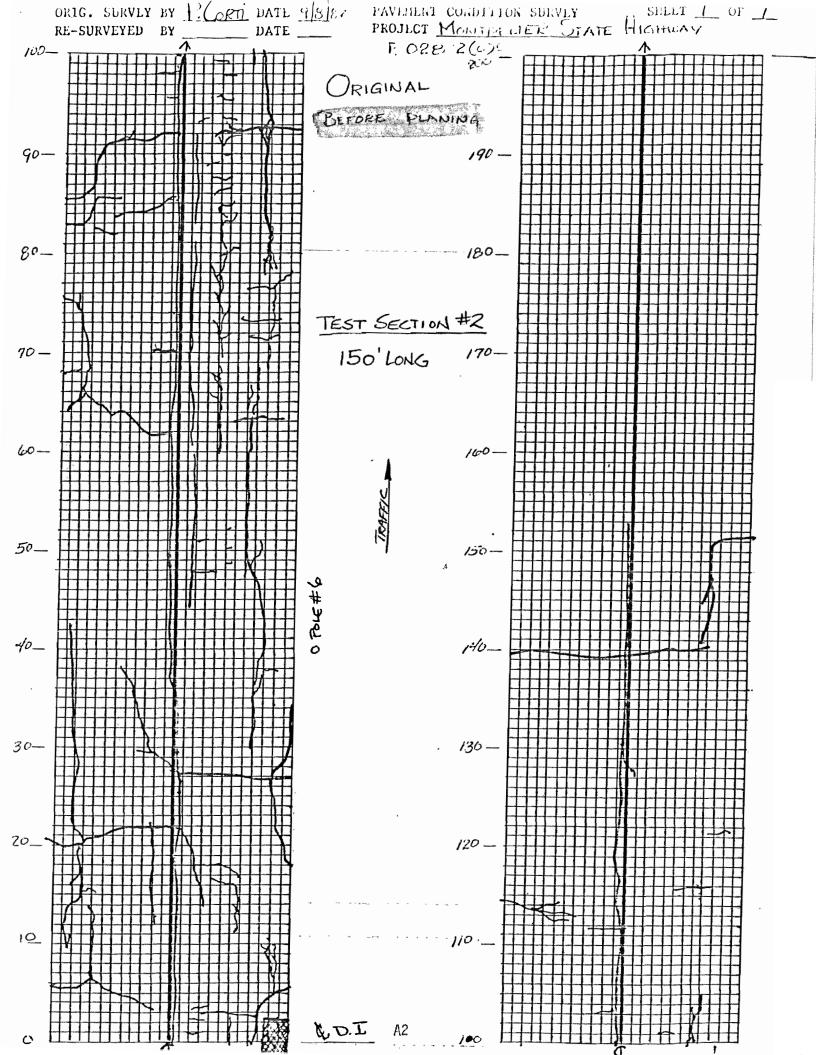
The long term performance of the modified pavement will continue to be monitored with emphasis on the following areas:

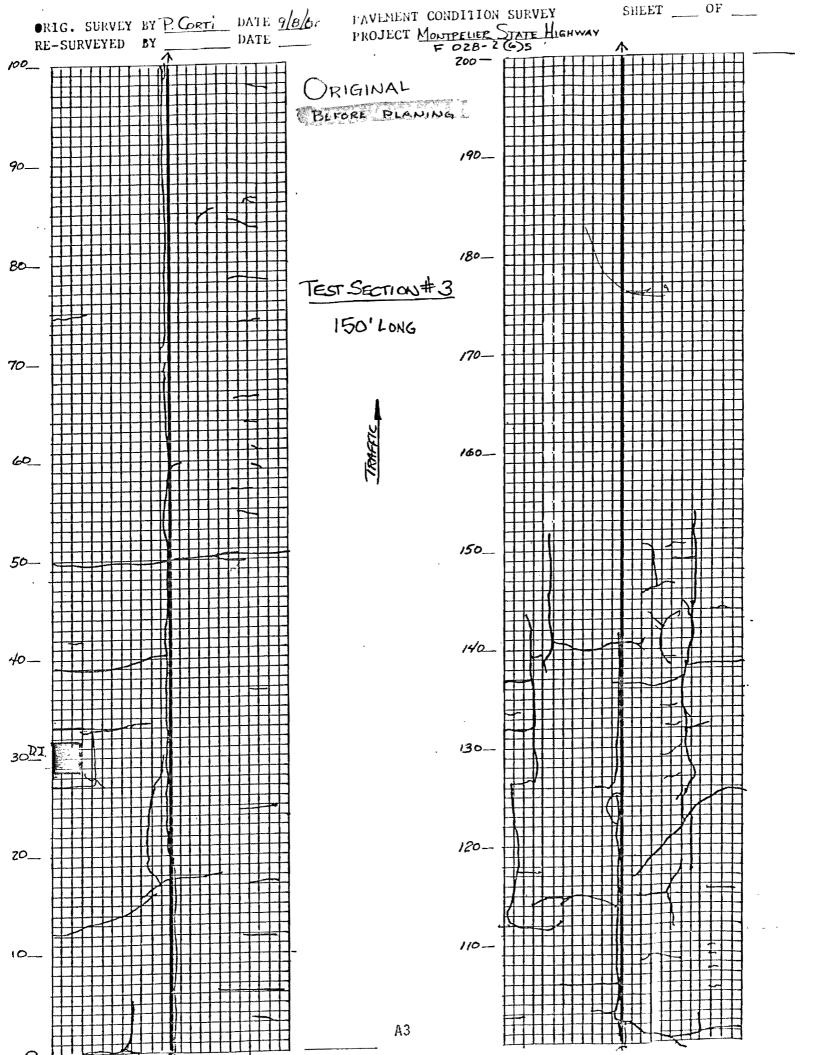
Reduction of reflective cracking.

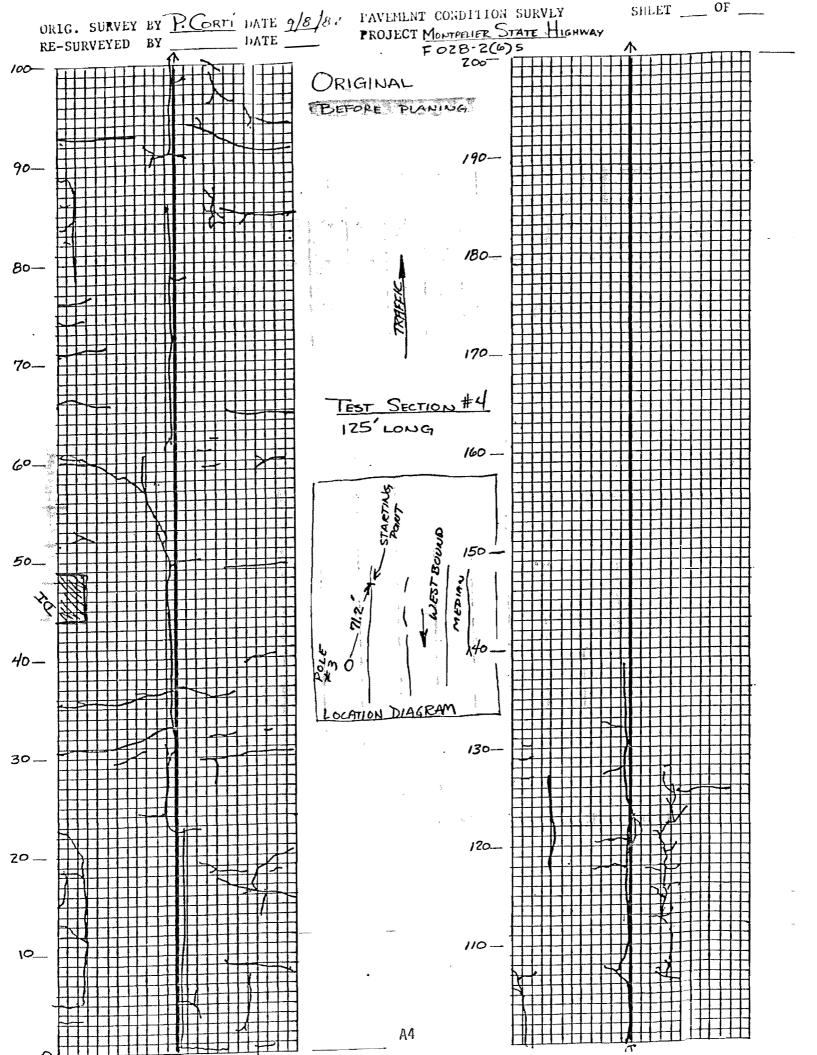
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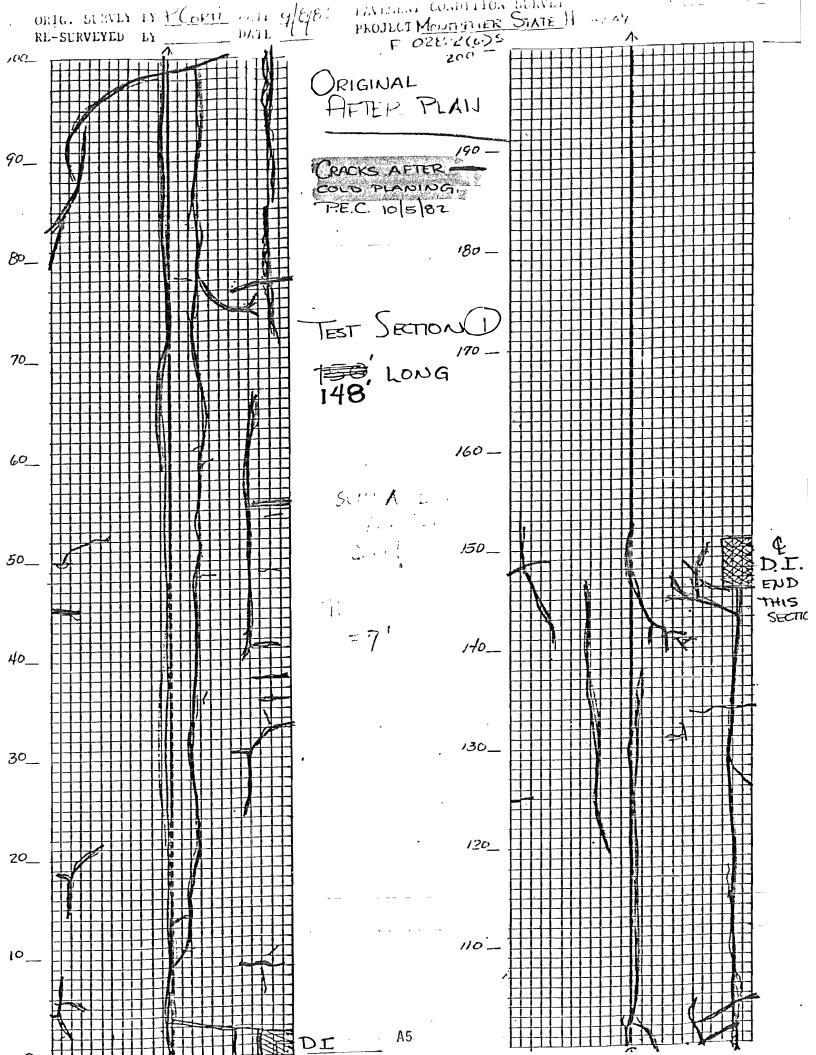
Recovered penetration values.

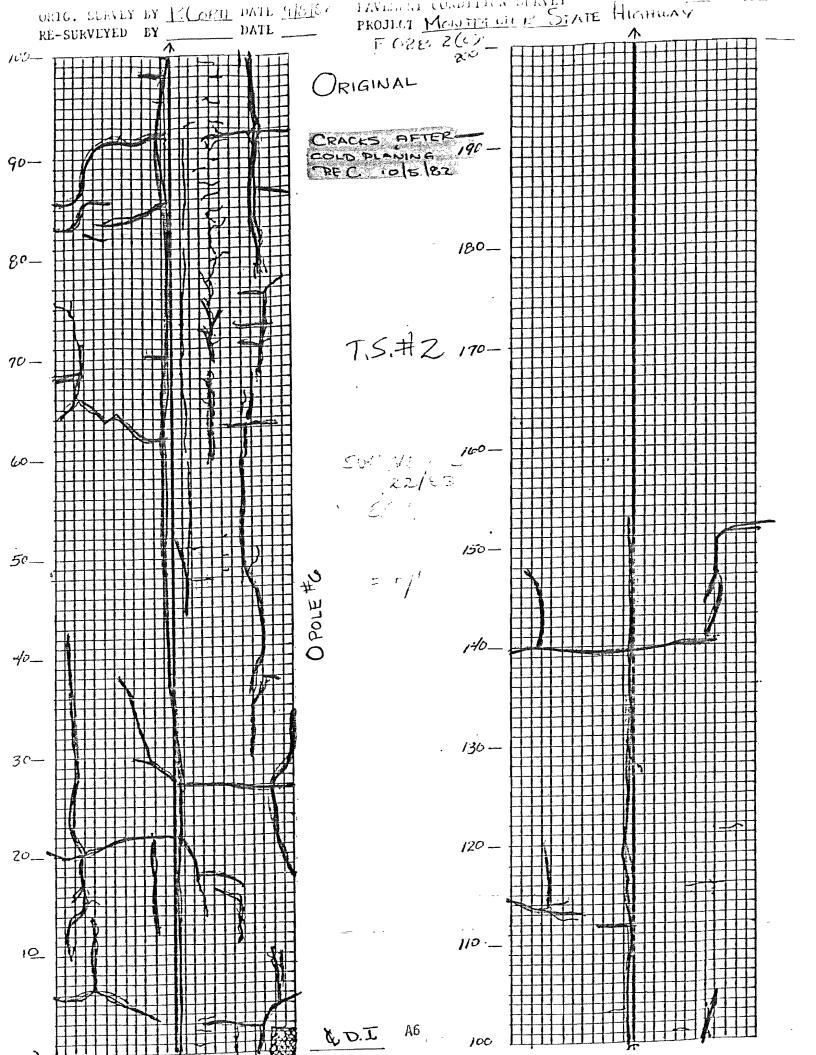


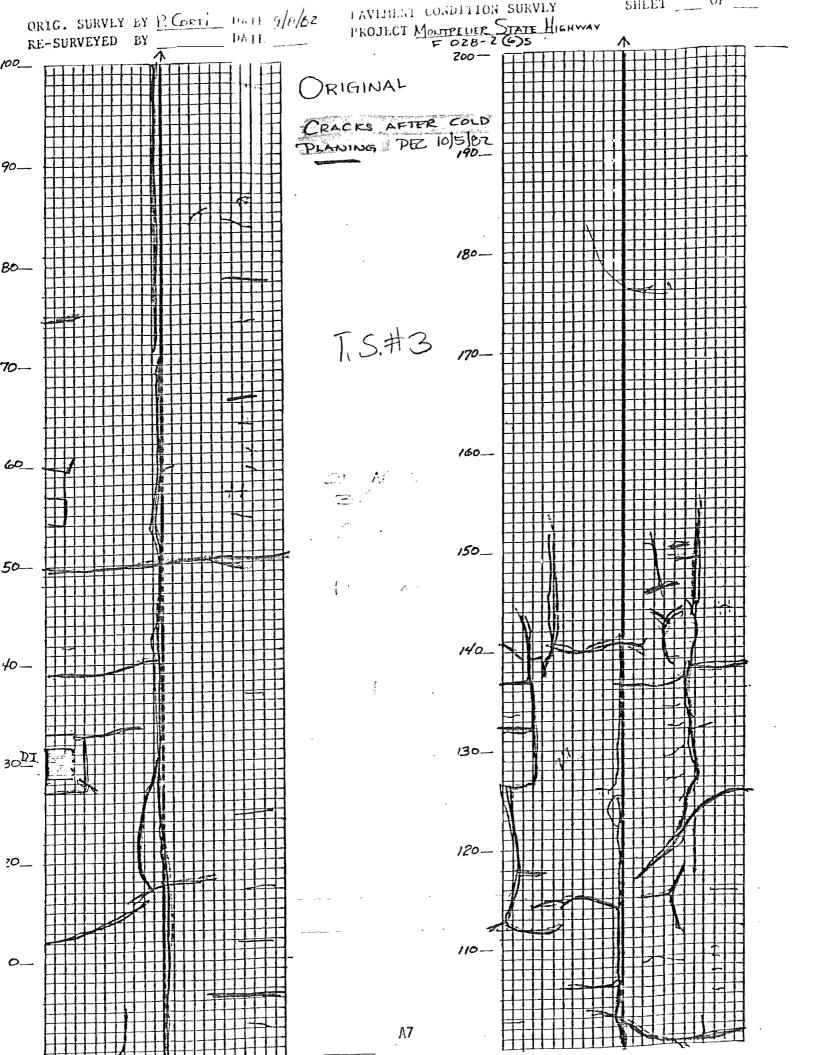


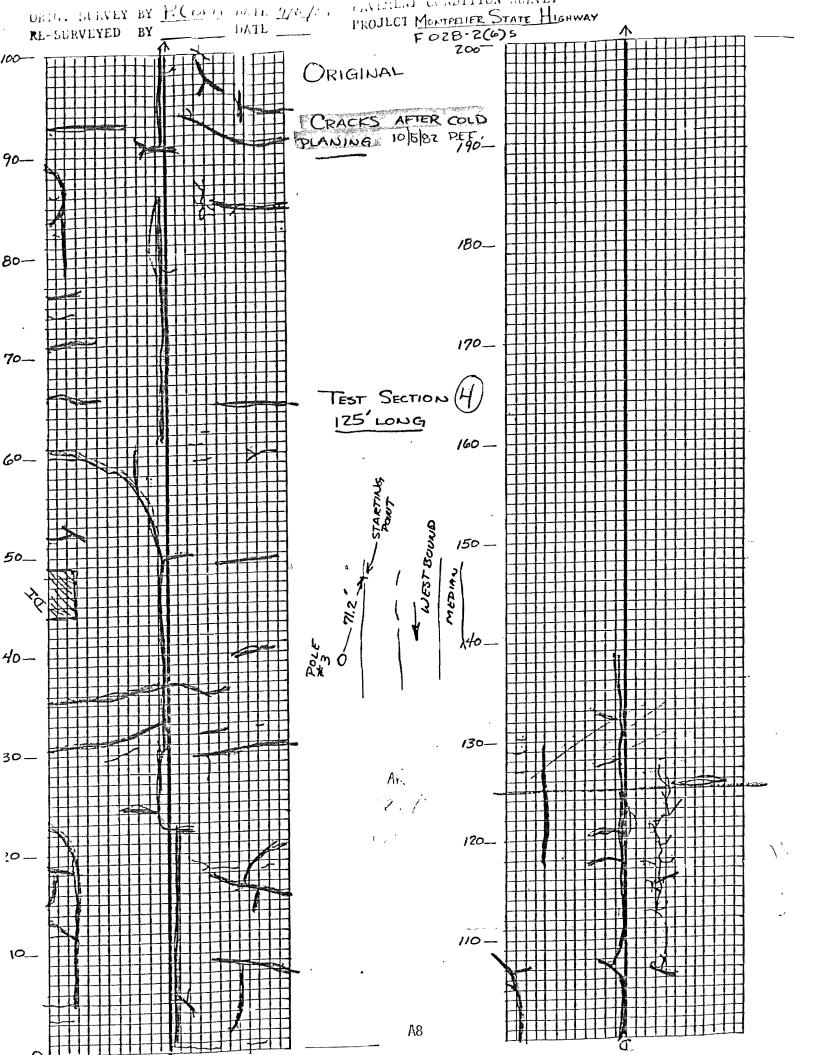












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#### STATE OF VERMONT

AGENCY OF TRANSPORTATION

MATERIALS & RESEARCH DIVISION - BITUMINOUS CONCRETE SUBDIVISION Nº

In accordance with the specification requirements for the above project I submit the following job mix formula:

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Project No. 1-028-2(6)S

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AGENCY OF TRANSPORTATION

MATERIALS & RESEARCH DIVISION - BITUMINOUS CONCRETE SUBDIVISION No Design of Bituminous Concrete Mixtures

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FIL Con For	uminous Conc E MAINTENANC trol No. H61 m No. 4 of m Date 10/1	E 6 6 /79	Materials ASPHALT - F: (It		ch Division ROPERTIES Data -		82	22600
Pro	ject Name 🖊	LONTPELIE	n_ ·		Projec	t No. F	028-26	۵ (م
1.	F	T-3	,					
1 .	cce of Mix 2		ET BE	RLIN	Ring l	10. 93	89 Ye	ar Bl
Item No.	406.25	Type of (	2		·	Sp	. Gr. AC	1020
(No	te- Begin a	new page if	any of the	he prior fi	elds chang	e)		
Line	Formula	Descrip	tion	1	2	3	4	5
A		Lab No.	D.8.2	1.9.18	1.9.1.9	1.9.2.0	1.9.2.1	1.9.2.2
В	-	Field Spec		1	, [2, τ	, ½3.Τ	,4,7	, .5.T
C	İ	Test Date		<del>. , - 1</del> 1	1011	1011	1.01,2	1,01,2
D3	Ì	% Passing	1"					
D4	100	11	3/4"	11.0.0.0	1,0,0,0	1,0,0,0	1,0,0,0	1.0.0.0
D5	95-100	11	1/2"	1,9.9.1	,9,8.0	,9,8,4	,9,9.7	,9,9,0
D6	78 - 90	11	3/8"	1841	,8,2,4	.8.1.5	7.9.7	,8,2,3
D7	56-68	11 -	#4	.6,1.8	6.0.1	6.0.5	15,6.4	.6.1.3
D8	43-51	11	#8	14.8.2			<del></del>	27-a
D9	31-39	11	#16	.3,5.8	, 3,4,6	,3,9,5	, 3, 3, 8	, 3, 5, 2
D10	20-28	***	#30	; 2,4.1	, 2,3 ,9	. 2.8.5	,2,3,4	, 2, 4, 3
D11	11-19	11	<b>#</b> 50	, 1,3,7	1.3.1	,1,6.5	,1,2,6	, 1, 3, 2
D12	2-5	11	<b>#200</b>	, , 2, 7	1.5.4	1.3.6	2.1	, ,2,4
E	6.0-6.7	Bitumen %	(AC)	.6.2,9	.6.2.7	,6,2,2	6.Z,9	.6.2.9
F	100(R-P)/R	% Voids - 1	Mix	, , 3,9	3.9	, ,3,7	, ,4,3	, 4.9
G	100S/(S+F)	" - ]	Filled	7.8.6	7.8.6	7,9.7	,7,6,9	. 7.4.3
Н	P x 62.4	Unit Wgt.,	1b/ft <sup>3</sup>	1,4,5,3	1,4,5.1	1,4,5,7	1,4,4,5	1,4,3,2
I	·	StabConv.	, 1b.	2,4,2,4	2,5,2,5	2, 4, 2, 4.		
J		Marshall F	Low Value		, , ], 2,	1.0.	, 1,3	, 1,5
К		Sample Thic	k. in	2.5,0,0	25001	12 5001	75638	7.5.0,0 /2
L L		Wgt. in Air		1,19,4	1,2,0,5	1, 2, 0, 5	1,2,2,3	1,2,2,3
M		Wgt. in Wat		6,8,1	68.7	,6,8,9	6.95	,6,9,1
N		Wgt. surf.		1.1.9.4	1,2,0,5	1,2,0,5	1, 2, 2, 3	1,2,2,4
P	L/(N-M)	Sp. Gr E		2.3,2,8	2,3,2,6	[	2.3.1.6	2.2,95
R		Sp. Gr N		2.4, 2,3	2.4.2,0	2,4,2,4	2.4.1.9	24,14
.S	PxE/SpGrAc	AC by Volum	<del>-</del>	1.4.3.6	1,4,2,9	1,4.5,2	1.4.2.8	1.4.15
T	-	StabMeasu	-		12.50	2,40	, Z, O, O.	2.5.5
U		Accept, Rej		Α	A	A	A	A
Field	i Notes:	· · · · · · · · · · · · · · · · · · ·		,, ,	· · · · · ·	spector(s):		me Staud:
	# 3T HA	S SOLAR	Laglugel	Additive	i —	Du Bois Prentiss	(3) Lie	
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							M "	ROSEM
				<del></del>			Rivers	1/80 (8)

#### STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH DIVISION

#### BITUMINOUS CONCRETE EXTRACTION WORK SHEET

	over ber	Foz			18 <sub>x</sub> 0	DATE MIX TYPE SAMPLE NO.		11-8 I 2T	
BOWL & MIX 2 BOWL 1 MIX 1 %AC = MIX - AGG =	243 243	PAI AG	N G.	3 	34 ,2	SLIP NO. TIME MIX TEMP.	9.	088 157 85°	
% Slip Ac = <u>6,27</u>	%	ļi	JE I	D ST	3	% STONE (+8 % SAND (-8) % AIR VOIDS	4	3	
RETAINED ON	WEIGHT	日	<b>%</b>	ETAINE	PASSING	JOB A	AIM		REPORTED
1 3/4		194	, C	2 8 8 x	j	l 		<u> </u>	
1:1/2		1	7 6	Mile /				<u> </u>	
1		<del></del>	<b>V</b>	(6)					
3/4					100	100	)		100
1/2	22		7	2.0	98.0	95-1			98
3/8	181			5.6	82.4	78 -0	70		82
4	259	4	2	2,3	60.1	56-	68		60
8	158		1.	3.6	46.5	43 - 3	s-1		47
16	138			1.9	34.6	31-	39		35
30	124		10	0,7	23,9	20-7	28	•	24
50	125		10	0.8	13,1	11-	19		13
200	124		10	.7	z.4	2 - 3	<u> </u>		2
Pass 200	28		2.	4					
Totals	1159								
					% Slip AC	6.0-6	, 7		6.3
	BIN S	BIN	NO. 2	BIN NO. 3	BIN NO. 4	BIN NO. 5	AC		TOTAL
BATCH WEIGHTS	3557	164	12	1360			441		7000
Wt. ADJUSTMENT									

CORRECTIVE ACTION

COMMENTS:

O.K.

P. Prentiss

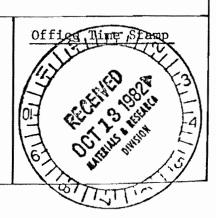
#### STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH DIVISION BITUMINOUS CONCRETE SUBDIVISION

#### MAXIMUM SPECIFIC GRAVITY WORKSHEET AASHTO T209-78 ASTM D2041-78

Design No. ZZ 60 Bulk Sp. Gr. Z, 3	
Flask No.	
1 UA of Elect 1 Comple	Z393
1. Wt. of Flask + Sample	
2. Wt. of Flask	1045
3. Wt. of Sample (1-2) (A)	<u> 1348</u>
4. Wt. of Flask filled with H <sub>2</sub> O (D)	3237
5. Wt. of Flask + Water + Sample (E)	<u>4028</u>
CALCULATION:	
Max. Sp. Gr. = $A/(A + D - E)$	
= 134 <i>B</i> =	2.420
1348 + 3237 - 4028	
% Voids Mix = 100 X Max. Sp. Gr Bulk Sp. Gr. Max. Sp. Gr.	<u>.</u>
2.420 - 2,326 =	3 <i>.88</i> 4

Comments:

Inspector(s)



#### STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH DIVISION

#### BITUMINOUS CONCRETE EXTRACTION WORK SHEET

SOURCE	ATPENER OLEY BERLI		,		-	DATE MIX TYPE SAMPLE NO.	<u> </u>	<u> </u>
	1327	PAI AG	N G.	3	73 34 39 63 %	SLIP NO. TIME MIX TEMP.	610 121 30	25
% Slip Ac = 6, 22	%	/-		WED SON	3	% STONE (+8 % SAND (-8) % AIR VOIDS	50	
RETAINED ON	WEIGHT	r (0	700	ETAIRED SI	PASSING	JOB A	AIM	REPORTED
1 3/4			63	Julia Richard	1:1			
1 1/2			V>	. ,	·/			
1				4.11.0				
3/4	-				100	100		100
1/2	14		/	1.6	98.4	95-10	Go	98
3/8	209		15	0,9	81.5	78-9	Q	82
4	761		1	1.0	60.5	56-6	8	61
8	126		1	0,2	50.3	43-5	1	50
16	134			0.8	3 <b>7</b> .5	31 <u>-3</u> 6	7	39
30	136			1,0	z8.5	20-2	8	Z <u>8</u>
50	149			2,0	16.5	11-19	7	17
200	160		١.	2.9	3,6	2-5		4
Pass 200	45		3	,6				
Totals	1234							
					% Slip AC	6.0-6.	7	6.2
	BIN S	BIN	NO. 2	BIN NO. 3	BIN NO. 4	BIN NO. 5	AC	TOTAL
BATCH WEIGHTS	3557	164	۲	1360	.,,,,,,,,		441	7000
Wt. ADJUSTMENT								
CORRECTIVE ACT	ION				COMMENTS:	25 K A)6	TE! SOU	AR LAGLUGEL

P. Prentiss
Inspector

# STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH DIVISION BITUHINOUS CONCRETE SUBDIVISION

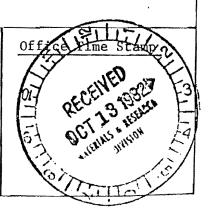
# MAXIMUM SPECIFIC GRAVITY WORKSHEET AASHTO T209-78 ASTM D2041-78

Design No. Z260 Bulk Sp. Gr. Z	. 335
Flask No.	
1. Wt. of Flask + Sample	2.395
2. Wt. of Flask	1045
3. Wt. of Sample (1-2) (A)	1350
4. Wt. of Flask filled with H <sub>2</sub> O (D)	3237
5. Wt. of Flask + Water + Sample (E)	4030
CALCULATION:	
Max. Sp. Gr. = $A/(A + D - E)$	-
= 1350 =	2.424
557 1350 +3237 -4030	
% Voids Mix = 100 X Max. Sp. Gr Bulk Sp. Max. Sp. Gr.	Gr.
2,424 - 2,335 =	3,672

Comments:

Inspector(s)

P. Frentis



#### STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH DIVISION BITUMINOUS CONCRETE SUBDIVISION

Plant Daily Report

					•	LIGHT A	Daily	Report			Par		<b>#</b> _	5
Proje	ect & N	0. N	ONTP	FIFE	Fo.	28 - 21	(6)5	***************************************			She	eet	1 01	<u>5</u>
	uood By egate S			REE	3CIN						Da <sup>*</sup>	te <u>10</u>		82
1,691	Conrse	_ <u>Coo</u>	L67.	-WEB	JEN VI	45				Weathe Air Te AC Sp.	r _	D664		
AO C.	Fine _	0 0	<u>.</u>		<u> </u>	<del>4</del> 7	ROAD	PIT		_ Air Te	mp_	25	_ AM	}
AC SC	ource _	ZP.				-	<u> </u>	3-730		THE SP.				
<u> </u>			E	TRACTI	ON RESU	LTS	γ			_		AGGREG/	TES	
Siev	re Aim	MZT	M3	<u>r  </u>			Aim			<b></b>	Coars	e Aggr	ega te	
1-34		]							1		 			
1-1/2										1-3/4				
?		1						1		1-1/2				
3/4	100	100	100		İ			1	<u> </u>	1				
1/2	95/00	98.0	198.	<u> </u>		1		1		3/4				
3/8	78 90	82.4	181.5	·   '				1	1	1/2				
#4	186	60.1	60.5	-				1		3/8			<u> </u>	
#8	43 51	465	50,3					7		#4				
#16	31 39	34.6	39.5	-	-			1		#8				
# <i>3</i> 0	20	23.9	28.5							T& E				
#50		13.1								FRAC			T	
#200	12/5	2.4	•					1		il .	Fine	Aggre	gate	
AC	12.0	6.5										1	T	
Slip A		6.3		İ	1 1	j		1	H			İ	İ	
	Voide	3.9	3, 7		İİ					3/4		.4	İ	
Slin N		6088	6106							1/2				
Time	_	9:57	12;25	1	İ	i		i		3/3		Ì		
				•		<u>.</u>	<u> </u>	•		#4				
	PF	ROJECT	TONNA	GE BY	MIX TYP	E	T	OTHER	7	#8				
		303			Type II	7	e IV			#16				
RIOR				OF	103	65			1	#30				
roday					674	10			┧ ┧	#50				
ΙΝΤΛΙ,	1		+		777	657			1	#200				
			1		1///	1401	11		_r \	]#280	!	!		
МIX	MIX T	IME	SP.	GR.	1 /	E	BATCH V	WEIGHTS	<u> </u>	_	-	FOR OF	FICE U	SE
Type	Dry		+ 8	1 - 8		Bin 2	Bin :	3   Bin	4   A		tal.			
Щ	5	35			3557	1642	1 360		49	11 700	0		1796	1
			_	7.5			\	1.1.		····				1
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								$\bot$				-	1 Marylane	
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TA 424	Re	v. 500						D.	<u></u>	J. 1	0 -	<i>t</i> .	deri	
		1M 6	/82					vas	ricia	J J. 1	nen	uss		

TNSPFCTOR

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JECT:	Martpe	len .F	028-2(	<b>-</b> ω>s	DATE	11/22/82	ITEM: 406 1-Leroz-CAC
40.	Station No., .	Offset	Lane	Depth	Tolerance	Remarks Reec	1-Leroz-CAC
1	0080.	5RF	NB	144 *	10-11	2-331	
2		5-L+	NB	114"	10-7		
3		584	SB	1/2"	10-12	2.306	
4		54	SB	1/2"	10-14	2-321	
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## STATE OF VERMONT AGENCY OF TRANSPORTATION MATERIALS & RESEARCH DIVISION BITUMINOUS CONCRETE SUBDIVISION

Date Prepared: 11-29-82

Prepared By: W. Royce

Montpelier Core Analysis
Project: F028-2(6) S

Type o	f Mix:	777				,				In Plac
Lab Number	Core No.	Weight(grams) Surf Air Water dry		Roadway Sp. Gr.	Plant Avg. Daily Bulk Sp. Gr.	Max. Sp.Gr.	Ratio	% Voids in Place	Unit Weight (p.c.f.	
A	В	С	D	E	F	G	Н	I	J	K
					C E-D			FG	H-F (100)	62.4xF
	,		26/1	612	2.284	2.331	2.422	98.0	5.7	142.
	0	500	370	590	2.257	2.328	2.423	47.0	6.4	
	3	747	418	748	2.264	2.306	2.417	98.2	6.3	141.3
	4	686	383	686	2.264	2.321	2.410	97.5	6.1	141.3
	·									
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							<i>a. o</i>			
				10	RAA	- 11-2	9-8	<u> </u>	у у	
						•				
					C1	7				

/ermont Agency of Transportatio... Materials & Research Division Bituminous Concrete Sub-division Report on Asphalt Cement

Sheet / of /

Prepared By: D. Day Date Prepared: 12/10/82

Project No.

AC Source

For Item

MONTPELIER FORB-2(6) 5

Plant Location Cooley-BERLIN AC Type

85/100 BP-Montraul

406

Year <u>1982</u>

Code for I.D. Column

Job Sample (J)

Preliminary Sample (P) Progress Sample (PR)

	101 100111									Progress Sample (PR)					
			ORI		RESIDUE										
Lá	ab No.	Date	I.D.	Abs. Vis.	Kin. Vix.	Pen.	Sp.Gr.	Loss Htg.	Abs. Vis.	Kin. Vix.	Pen.	A/R			
B82	1796	195/82	J	1377		90						A			
	1808	195/82	J	1290		88						A			
	1809	195/82	V	1321		91						A			
B8Z.	1810	195/82	U	1307		92						A			
B82	1811	196/82	J	1242		87						A			
<i>B8</i> 2	1869	197/82	V	1245	316	88						A			
<i>B82</i>	1880	10/7/82	J	1202		93						A			
<i>B8</i> 2	1881	10/7/82	V	1308		87				<u> </u>		A			
	1871	198/82	U	1348	328	88				<u> </u>		A			
<i>88</i> 2.	1878	10/8/82	J	1325	327	86						A			
B8Z	1879	198/82	J	1256		90						A			
<i>B82</i>	1833	19/1/82	J	1369		85						A			
<i>B8</i> 2	1834	19/1/82	U	1426		21						R			
882	1203	19/1/82	J	1203		90						A			
<u>882</u>	1847	19/12/82	J	1356		86						A			
B <i>8</i> 2	1868	19/12/82	V	1274		93	!					A			
BE2_	1870	19/2/82	V	1336		90						<u>A</u>			
				1			1								
		1		<u> </u>											
											<u>[</u>				
											į				
-															

Prepared By: R. I. Frascoid A., Date: August 26, 1982

Page 1 of 3

STATE OF VERMONT
AGENCY OF TRANSPORTATION
MATERIALS & RESEARCH DIVISION

WORK PLAN FOR CATEGORY II EXPERIMENTAL PROJECT

RESIN MODIFIED ASPHALT PAVEMENT

WORK PLAN 82-R-19

#### OBJECTIVE OF EXPERIMENT

To produce and place a bituminous concrete mix modified with a natural and synthetic resin admixture and to compare the properties and performance of the modified material with a standard bituminous concrete mix.

#### **PROJECT**

Montpelier F 028-2(6)S

#### PROJECT LOCATION

In the City of Montpelier, on the Montpelier State Highway, beginning at the intersection of the Montpelier State Highway and Ramp A and extending easterly 3981 feet, ending 100 feet west of the intersection with Bailey Avenue Extension.

#### EXPERIMENTAL WORK LOCATION

On 2900  $\pm$  lineal feet of 12 foot wide roadway beginning near the intersection with Bailey Avenue and continuing westerly. The work location is subject to change, depending on mix production and field conditions.

#### MATERIALS TO BE USED

The experimental mix shall be modified with SOLAR LAGLUGEL, a compound consisting of a nylon gel and 14 natural and synthetic resins. The material is available from Solar Asphalt of America, Inc., 1361 St. Georges Avenue, Rahway, N.J. 07065. Phone (201) 381-5522.

#### PRODUCTION PROCEDURE

The SOLAR LAGLUGEL Modifier shall be added to the asphalt cement in the delivery tanker or in a storage tank at the batch plant at the rate of 1.33 percent by weight of the asphalt cement. Mixing shall be achieved by circulating the mixture for 60 minutes at temperatures between 280°F and 320°F. Approximately 325 tons of the modified mix shall be produced.

#### CONTROL SECTION AND TREATMENT

The control section shall consist of adjacent areas of bituminous concrete pavement constructed with the same bituminous mix without the admixture. Sufficient data will be gathered on the control section to make the desired comparisons with the modified pavement section.

#### COST

The in-place cost of the standard bituminous concrete pavement shall be \$26.50 per ton. There will be no increase in cost for the modified mix, due to the cooperation of the material supplier and paving contractor.

#### DATE OF CONSTRUCTION

The experimental treatment shall be completed prior to October 15, 1982.

#### EVALUATION PROCEDURE

The initial evaluation will include the following steps:

- 1) Obtain initial design, construction and maintenance records on the section of highway which is to be overlaid.
- 2) Visually inspect and document the condition of the existing pavement.
- 3) Observe the production and paving process and document pertinent information on the equipment required, method of production, mix temperatures, compaction effort required and achieved, weather conditions, and other related information.
- 4) Document laboratory and field tests taken during the construction of the project and obtain core samples of the standard and modified pavement for lab analysis.
- 5) Obtain Mays Meter ride values and friction tests on the experimental and control sections.

#### DURATION OF STUDY

The experimental project will be evaluated for a minimum of five years following completion of construction.

#### SURVEILLANCE

The experimental and control pavements shall be monitored during construction and at least once each winter and spring for the duration of the study. Evaluations shall include documentation of the condition of both experimental and control pavements. The long term performance of the modified pavement will be compared with that of the standard pavement with emphasis on the following areas:

- 1) Reductions in reflective cracking.
- 2) Retention of initial ride values.
- 3) Retention of initial friction values.

#### REPORTS

An initial report covering the production, placement, and initial observations and a final report showing conclusions on the effectiveness of the experimental material shall be submitted to the Federal Highway Administration.

Reviewed By:

R. F. Nicholson, P.E.

Materials & Research Engineer

Date: <u>Sept. 3,1982</u>

D.J. Nicholan

Materials & Research Division Agency of Transportation August 31, 1982