### STRUCTURAL CONCRETE SUBDIVISION

January 1976

LOW STRENGTH CONCRETE INVESTIGATION

### INTRODUCTION

Portland cement concrete batched in Galkins Redimix facilities at Lyndon, Vermont has occasionally failed to comply with Vermont Highway Department strength specifications. This investigation was initiated to determine the probable causes underlying the problem. Component materials are examined to determine if any are adversely affecting the concrete.

#### MATERIALS

Following are listed the materials used in this investigation and their sources. Each of the materials listed meet current Vermont Highway Department and AASHTO Standard Specifications:

Aggregates: (See Chart I)

Coarse and Fine

Caledonia, Inc. Waterford, Vermont

Cement:

Type I

Northeast Cement Co., Inc. Montreal, Quebec

Admixture:

NVX

Hercules Powder Co. Wilmington, Delaware

#### PROCEDURE

The procedures used in mixing followed as closely as possible that required by AASHTO T-126 and every effort was made in controlling uniformity of detail.

All batches were mixed in a Lancaster pan-type mixer.

Two series of investigations were made. The first was a program to ascertain if field procedures were responsible in affecting the specimens. Laboratory mixes using typical field proportions and materials were prepared and tested as shown on Chart II.

The second investigation was made to observe the affect of the aggregate upon the mix. Previously, it had been suggested that crusher dust or other deleterious matter was causing loss of bond. (See Appendix A.)

Mixes were made incorporating aggregate from the production stockpile. However, half of the aggregate was further cleaned by washing out the fine particles. Charts III and IV show the mixes and results of this investigation. Note that Mixes 4A, 4B, 6A and 6B incorporate hand washed aggregate.

A comparison was also made between mixes containing 3/4" and 1-1/2" maximum size aggregate. Previously, it had been suggested that the 1-1/2" mix (Class BB) was too coarse (see Appendix B).

#### **RESULTS & COMMENTS**

Compressive strength results from both investigations were below expectations at every age of test.

The results of the first part of this investigation, shown on Chart II, illustrate that higher strengths can be expected when using smaller maximum size aggregate. Note that Class A achieved higher strengths than Class BB, even though its water-cement ratio was higher. Also, the addition of one-half bag of cement resulted in a negligible increase in compressive strength as shown when comparing Class B with Class BB Concrete mixtures.

The results of the second part of this investigation are shown on Chart III (Class BB) and Chart IV (Class A). Mixtures containing the hand-washed aggregate required slightly less water with a corresponding increase in strength. The wash water was chemically analyzed for sulfate content and found to be inert. As in part I, the Class A mixtures incorporating smaller aggregate resulted in higher strength concrete.

Generally, the 3, 7 and 14 day tests were extremely high in relation to the 28 day strengths. In fact, several 14 day specimens broke higher than their companion 28 day cylinders. The tests indicated that the concrete gained strength very slowly after about 14 days.

The air-entraining admixture "NVX" was substituted for Grace Chemical's "Darex" which is typically used in the field. No effect on strength was observed when comparing laboratory results with project records. Compressive strength of the concrete was lower than normal for the mix design at 28 days.

3/4" maximum size aggregate resulted in higher strengths than 1-1/2" maximum size aggregate when compared in mixes with cement contents of 6-1/2 sacks.

Increasing cement did not increase concrete strength proportionately.

The aggregate, as received from the crusher plant, was found to have little effect on the concrete strength when compared to hand washed aggregate.

Strength gain of the cement appeared abnormally high at early ages for Type I cement.

All materials, including cement, separately met applicable standards of acceptance.

In my opinion, the cement was deficient even though the mill tests showing its chemical composition and our laboratory tests for determining physical characteristics all were acceptable. It is conceivable that the specifications followed by the industry are too broad or do not apply to the current problems. These problems have recently been magnified by environmental restrictions and regulations imposed on the cement manufacturing industry.

I recommend that dependence on standard acceptance tests be continued.

Additional tests should be developed for early determination of concrete strength. Such a test could be ASTM C684-74, "Making, Accelerated Curing, and Testing of Concrete Compression Test Specimens".

Furthermore, I recommend the purchase of the parts needed to rebuild and make operational, the concrete coring machine. It would allow us to monitor the increase in strength that normally occurs with age.

# VERMONT DEPARTMENT OF HIGHWAYS MATERIALS DIVISION

# Structural Concrete Aggregate

SOURCE				PREDOMINANT MINERAL COMPOSITION					
1-1/2" Stone Caledonia, IncWaterford				Gr	avel 1	Deposi	t		
3/4" S	tone <u>Caled</u>	onia, IncW	aterford				the second s		
Sand	Caledonia,	IncWaterf	ord, Vt.						
SIEVE	1-	1/2" STONE				;	3/4" STO	NE	201 p. t. 2000 p. p.
SIZE	CUM.% RET'D	% PASSING	SPEC.	CUM.% F	RET'D	% PAS	SSING	SPEC.	
1-3/4" 1-1/2" 1" 3/4" 3/8" No. 4 No. 8 F.M.		100 98 53 10 2	100 90100 20 55 0 15 0 5	7 9	2 6 7		00 98 24 3 2	100 90-100 20- 55 0- 10 0- 5	
Absorp Passin T & E Fractu Wear ( Soundn	ic Gravity tion, % g #200 Sieve Pieces, % red Faces, % AASHTO T96), ess Loss, %	, % 0. 7	8 6 6 3	Specifi Absorpt Passing T & E H Fractur Wear (A Soundne	tion, #200 Pieces red Fa	% ) Sieve 5, % aces, % ) T96) pss, %	% , %	2.91 1.0 1.5 6 94 25.3 2.8	
SIEVE	CUM.% RET'D		SPEC.	SIEVE	CUM.9			CUM.% RET.	% PASS.
3/8" #4 #8 #16 #30 #50 #100 F.M.	14 35 57 79 91 2.76	100 86 65 43 21 9		1-1/2" 3/4" 3/8" #4 #8 #16 #30 #50 #100 F.M.		1 37 56 61 67 75 84 92 92	99 63 44 39 33 25 16 8 3	48 58 65 74 83 92 96	99 54 42 35 26 17 8 4
Absorp Organi Passin	ic Gravity tion, % c Color No. g #200 Sieve ess Loss, %			BLEND: 38 21 41	% Sa % 3/	nd '4" Sto 1/2" S	one Stone	40 % Sa 60 % 3/ % 1-	nd '4" Stone 1/2" Ston

Remarks;

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## VERMONT DEPARTMENT OF HIGHWAYS

### MATERIALS DIVISION

# Structural Concrete - Mix Proportions and Results

Mix Number	_1	2	3	 
Mix Design:				
Class of Concrete	A	В•	BB	
Cement - 1bs.	611	564	611	
1-1/2" Stone (SSD) - 1bs.		1395	1400	
3/4" Stone (SSD) - 1bs.	1971	702	690	
Sand (SSD) - 1bs.	1264	1242	1224	
Net Water - 1bs.	306	273	280	
Admixture - oz. NVX	1.7	1.6	1.7	
Admixture - oz. Total Batch Weight - 1bs.	4152	4176	4205	
Concrete Tests Water/Cement Ratio	.50	.48	.46	
Slump - Inches	3	3	3	
Air Content - % (Chace) Air Content - % (Pressure)	6.5	6	5.3	
Temperature - Deg. F	148.8	152.1	152.1	
Density - 1bs./cu. ft.	27.9	27.4	27.6	
Mix Yield - cu. ft.	591	555	597	
Calc. Cement Content - 1bs.	791	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	271	
Compressive Strength - PSI				
7 Days	2476	2299	. 2440	
	2476	2440	2387	
Average	2476	2370	2414	 
14 Days	31.30	2829	3086	
Days	3006	2794	2918	
Average	3068	2812	3002	
28 Days	3431	2927	3201	
	3519	2882	2909	
Average	3475	2905	3055	 
Days		•		
Average				 
Days				
•		L-1-1-1-1-1-1		 
Average				

NOTES:

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#### VERMONT DEPARTMENT OF HIGHWAYS

### MATERIALS DIVISION

# Structural Concrete - Mix Proportions and Results

Mix Number	* 44	* 4B	<u>5A</u>	5B	
Mix Design: Class of Concrete Cement - 1bs. 1-1/2" Stone (SSD) - 1bs. 3/4" Stone (SSD) - 1bs. Sand (SSD) - 1bs. Net Water - 1bs. Admixture - oz. NVX Admixture - oz. Total Batch Weight - 1bs.	BB 611 1400 690 1224 269 14.8 4194	BB• 611 1400 690 1224 271 1.8 4196	BB 611 1400 690 1224 289 1.8 4214	BB 611 1400 690 1224 285 1.8 4210	
Concrete Tests Water/Cement Ratio Slump - Inches Air Content - % (Chace) Air Content - % (Pressure) Temperature - Deg. F Density - lbs./cu. ft. Mix Yield - cu. ft. Calc. Cement Content - lbs.	.44 3-1/4 7 5.4 149.1 28.1 587	.44 3-1/4 6 5.4 150.2 27.9 591	.47 3-1/2 6.3 5.4 147.7 28.5 578	.47 3 5.3 4.5 150.5 28.0 590	
Compressive Strength - PSI Days	2175	2299	. 2087	2210	
Average 7 Days	2290	2493	2387	2484	
7 DaysAverage14 Days	2918	3165	2803	2820	
Average	<b>DECOMPANI</b>				
28 Days	2989 2865	2829 2918	2383 2723	3006 2997	
Average	2927 3448	2874 3369	2553 3298	3002 3298	
Average		•		(Annual States	

NOTES:

\* Laboratory washed aggregates

#### VERMONT DEPARTMENT OF HIGHWAYS

### MATERIALS DIVISION

### Structural Concrete - Mix Proportions and Results

Mix Number	* 6A	* 6B	7A	7B	
Mix Decim		0			
Mix Design:	٨	1.	٨	A	
Class of Concrete	A		A		
Cement - 1bs.	611	611	611	611	
1-1/2'' Stone (SSD) - 1bs.			- 0.04-	1.001	
3/4" Stone (SSD) - 1bs.	1971	1971	1971	1971	
Sand (SSD) - 1bs.	1264	1264	1264	1264	
Net Water - 1bs.	261	268	288	276	
Admixture - oz. NVX	1.8	148	1/.8	1/.8	
Admixture - oz.					
Total Batch Weight - 1bs.	4107	4114	4134	4122	
Concrete Tests					
Water/Cement Ratio	.43	.44	.47	.45	
Slump - Inches	2-3/4	3	2-1/2	3-1/4	
Air Content - % (Chace)	6-3/4		6	6-3/4	
Air Content - % (Pressure)	5.7	5.4	5-1/2	6.8	
Temperature - Deg. F	2.1	2.4	)-1/c	0.0	
Density - 1bs./cu. ft.	147.3	146.6	147.2	142.8	
Mix Yield - cu. ft.	27.9	28.1	28.1	28.9	
Calc. Cement Content - 1bs.	591	588	587	571	
Compressive Strength - PSI					
3 Days	2405	2440	2255	1945	
Arrows go				. <del></del>	<del></del>
Average					
7 Days	1945	2476	2449	2334	
Days					
Average					
14 Days	3121	3050	3156	2546	
,					
Average					
28 Dava	3192	3519	3263	2759	
28 Days	3156		3183	2873	
	3190	3537	2102	2013	
	01.01	0504	2000	003	
Average	3174	3528	3223	2816	
60 Days	3563	3590	3625	3280	
Days	,,,,,	1110	//	,	
Average		* <u>p=0+1+1+1+1+1+1+1</u>			

#### NOTES:

\* Laboratory washed aggregates

APPENDIX A

# COMPANY INC.

August 29, 1975 NECEIVED SEP 3 1975 HIGHWAY HIGHWAY

Mr. Dave Bartlett, P.E. Concrete Engineer Materials Division Department of Highways State of Vermont Montpelier, Vermont

Dear Mr. Bartlett:

NORTHEAST

Luss-RB 9/4-

Earlier this summer, we agreed to run a series of comparative tests, using a Caladonia  $1\frac{1}{2}$ " stone with your standard class "BB" mix design.

Enclosed is a copy of my letter to Paul Calkins, discussing the results of our tests.

Please note that your findings do confirm points a, c, and d, but do not confirm point b. As mentioned, over the telephone, I would be very happy to continue looking into this discrepancy, should you so desire. Please contact me should you wish to further discuss this subject.

Yours sincerely,

Jan R. Bolo.

Ian R. Poole, P.E. Manager - Technical Services

ff

- cc Mr. Paul Calkins
  - Mr. Hubert Lavigne
  - Mr. Colin Barnett
  - Mr. Victor De Benedictis
  - Mr. F. T. Sendker

PAY



# COMPANY INC.

August 29, 1975



Mr. Paul R. Calkins Owner Calkins Ready-Mix Concrete Lyndonville, Vermont 05851

Dear Mr. Calkins:

NOFTHEAS

I have recently received from Francon the test results for both the aggregates used in your class "BB" concrete supplied to the State of Vermont, as well as the compressive strength results of the concrete when used in a class "BB" mix design.

The results with my comments are as follows:

a) The sand and the stone graduations in regard to both finances modulus and blending are good. (Refer to the enclosed data sheets.)

b) The compressive strengths for 7 days averaged 2725 p.s.i. while the 28-day average was 3945 p.s.i.

The 28-day strengths are between 500-1000 p.s.i. lower than what we would normally expect. (I would like to comment more about the cause of this problem in section (d).)

c) The yield for this mix was found to be 28 cubic feet per yard which is high. As a result of this, the actual class "BB" mix design per yard was:

Cement	-	588 lbs
Sand	-	1154 lbs
Stone 12"	-	1336 lbs
3/4"		652 lbs
Water	-	296 lbs (35.7 gallons)
Total Weight	-	4026 lbs
Unit Weight		149.1 lbs/c.f.
Air	-	5.5% (Using 5.7 oz of Darex)
Slump	-	3"
Temperature	of Concre	te: 76° ·



the state tower building • syracuse n.y. 13202 • telephone : 471-2551

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# COMPANY INC.

Mr. Paul R. Calkins August 29, 1975 Page 2

NORTHEA

d) According to our Laboratory, "there was not one stone broken in the two 28-day cylinder breaks and the top size aggregate could be removed freely by hand in most cases. The only apparent reason for the low compressive strengths is the very poor bond of the coarse aggregate to the Matrix, especially the large  $1" - 1\frac{1}{2}"$  size particles".

It is the belief of our Laboratory that the dust coating on the coarse aggregate is the cause of your problem and that this would also explain why approximately two gallons more water, was required than normal, to produce a 3" slump concrete. This basically confirms what I mentioned to you during my visit of July 14. It will also explain why you have been getting good results with your own aggregate through tests with Knight Engineering, while you were getting lower results with the Caladonia aggregate.

Yours truly,

n N. Role

Ian R. Poole, P.E. Manager - Technical Services

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Enclosures cc F. T. Sendker Victor De Benedictis Hubert Lavigne Colin Barnett



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# FRANCON LIMITEE

#### AGGREGATE GRADING ANALYSIS REPORT

Sample description: 11/3 Stone

Sampling location: Calkins Reade, Yiz Date: July 15 Ed 197 V

INDIVIDUAL SIEVE WEIGHT SPECIFICATIONS N Y er.-15s. 3 PASSING RETAINED SIZE 311. 212 2 100.0 1.7 97.8 132 3.2 2.2 45.0 -47.2 3 34.3 52.8 42.1 10.7 89.3 36 13,1 1 95.5 4.1. 4.7 6.2 22 . 0.9 1.2 3.3 96.7 3/8 .0.5 2.8 97.2 0.4 36 17.5 a.V No. 4 0.3 0.2 8 16 30 . 50 100 200 11 Sil 100.0 Pan Total 75.6 160.0 Unit weight, 1bs. per cu. Ft.: Loose Dry \_\_\_\_ Compacted Dry - Moist Moist

Finess	Modulus:
Hoistu	re, %:
Remarks	3 :

Yested by:

Checked by:

AGGREGATE GRADING AMALYSIS REPORT

S	ample descri	ption:	4 5.	one	
5 D:	ampling loca	tion: Con July 15th	<u>lyins</u> K 19	<u>Peadi. 1</u> 97 <u>5</u>	tix
SIEVE SIZE	UEIGHT gx-lbs.	INDIVIDUAL %	% PASSING	% RETAINED	SPECIFICATIONS
30.	-				
2%	-				
. 2	-				
1.72	-				
1			100.0		
×	3.7	1.1	94.5	5.5	
32	36.3	38.7	55.6	44.4	
3/3	34.5	16.3	19.3	80.7	
3%	11.3	16.7 .	2.6	97.4	
No. 4	0.7	1.0	1.6	98.4	an and a second a second of the second second differences in the second s
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Pan	1.1	1.6		100.0	
Total	67.7	100.0			941444 442 445 446 <b>4</b> 46 49 49 44 49 47 49 49 49 49 49 49 49 49 49 49 49 49 49
Finess H Molsture	ght, lbs. ps odulus: , រៈ		Leose Dry Mois	t	pected Dry Moist
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APPENDIX B

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DH.B



STATE OF VERMONT DEPARTMENT OF HIGHWAYS' MONTPELIER 05602

December 4, 1975

Mr. Thomas M. E. Mindock Chief Chemist The Flintkote Company Glen Falls Cement Division Glen Falls, New York 12801

Dear Tom:

Several months ago you sent me a copy of a letter that you received from the Portland Cement Association, dated May 22, 1975, which analyzed and commented on a mix design from Lawrence Sandgravco. The author, Mr. Mike Pistilli, Research Chemist, noted that the combined FM was high -6.77 and suggested that for aggregate of  $1\frac{1}{2}$ " top size, an FM of approximately 6.0 would be more ideal.

In reviewing this data, we have been unable to duplicate these figures. In fact, we have researched in NRMCA pamphlets, as well as several text books, to ascertain that our methods were correct. In every instance, using aggregate sources from throughout the State, we have found the combined FM to be less than 6.0.

We appreciate the opportunity to study your report and have adjusted some of our mix proportions as a result. However, we are puzzled as to the method and result of the combined FM and would welcome any further information in this regard.

Thanks again for forwarding a copy of the Portland Cement Association's comments.

Sincerely,

R. F. Nicholson Materials Engineer

By: . Ered

D. H. Bartlett Structural Concrete Engineer

RFN/DHB/msd cc: RFN/Lab File Central Files

Vo further information received as of 3/15/76 DAB

# PORTLAND CEMENT ASSOCIATION

Old Orchard Road, Skokie, Illinois 60076 / Area Code 312 / 966-6200

Research and Development Construction Technology Laboratories



May 22, 1975

Mr. Thomas M. E. Mindock Chief Chemist The Flintkote Company Glen Falls Cement Division Glen Falls, New York 12801

Dear Tom:

The following are some comments regarding concrete mix designs from Lawrence Sangravco Co., resulting in low concrete compressive strengths. The mix designs were attached to your letter of April 28, 1975 to Mike Meyer. You had previously discussed this with Mike by telephone.

The material finer than the No.200 sieve may be too high in the sand. ASTM allows a maximum of 3-7% dependent upon type of construction, the amount of clays and shales in this fraction, and if the sand is a crushed product. The major problem is with clays and shales, which increase the w/c ratio when dispersed into the paste. If the clays are not cleaned away from the aggregate surface, the paste to aggregate bonding is interferred with. This was evident in mix No.60 compared to mix No.61.

From the cement content of the mixes, I assume the design is for approximately 4000 psi at 28 days.

I also noticed that the combined fineness modulus (including sand and coarse aggregate) is high, 6.77. This would indicate that the mix is harsh. A good combined F.M. would be approximately 6.0 for 1-1/2" top size aggregate. Harsh mixes are coarse in appearance, and very sensitive to w/c for texture. They are also more variable in compressive strength than leaner mixes. The coarse aggregate blend used was 32.3% (3/4") and 67.7% (1-1/2") crushed stone. The mix design used was approximately 37% sand and 63% coarse. The mix may be improved by using 50% (3/4"), 50% (1-1/2") coarse aggregate blend and approximately 40% sand, 60% coarse. (This is provided the mix is not architectural concrete). Mr. Thomas M. E. Mindock May 22, 1975 Page -2-



This should lower the F.M. of the mix and still meet ASTM specification for gradation of 467 (C-33) coarse aggregate. This may also improve the strength and reduce variation.

Should additional questions arise, please feel free to contact us in regards to them.

Sincerely yours,

Mike Pistilli Research Chemist

HA1153/4140

Copies to -W. E. Kunze E. Hognestad J. J. Shideler/L. M. Meyer

### LAWRENCE SANGRAVCO - CONCRETE AGGREGATES - FEBRUARY 20, 1975

SIEVE		CONCRETE SANI	D	3/4" CRUSHED STONE				
SIZE	% RET'D	CUM. % RET'D	% PASSING	% RET'D	CUM. % RET'D	% PASSING		
1" 3/4" 5/8" 1/2" 3/8" No. 4 8 16 30 50 100	0.0 0.0 13.4 21.1 24.0 21.3 11.5	0.0 0.5 13.9 35.0 59.0 80.3 91.8	100.0 99.5 86.1 65.0 41.0 19.7 8.2	0.0 6.5 14.9 15.5 27.0 29.7	0.0 6.5 21.4 36.9 63.9 93.6	100.0 93.5 78.6 63.1 36.1 6.4		
DRGANIC SPECIFIC ABSORPTI MATERIAL	C COLOR NO. CLI IC GRAVITY 2. TION % 0.3 AL FINER THAN NO. 200		2.80 CLEAR 2.77 0.84 4.33		2.78 0.92 1.15			

SIEVE	1-1/2	" CRUSHED STO			6 3/4"&67.7% 1-	1/2" CR. STONE
SIZE	% RET'D	CUM. % RET'D	% PASSING	% RET'D	CUM. % RET'D	% PASSING
1-1/2"	0.0	0.0	100.0	0.0	. 0.0	100.0
1"	52.5	52.5	47.5	35.6	35.6	64.4
3/4"	36.3	88.8	11.2	26.6	62.2	37.8
5/8"	6.0	94.8	5.2	8.9	71.1	28.9
1/2"	1.3	96.1	3.9	5.9	77.0	23.0
3/8"	1.1	97.2	2.8	. 9.4	86.4	13.6
No. 4	0.6	97.8	2.2	10.0	96.4	3.6
TOTAL	97.8	-	-	96.4	-	-
SPECIFIC	GRAVITY		2.88		2.85	
ABSORPTI			0.72		0.78	
	FINER THAN					
	WASHING -		0.89	1	0.97	
DENSITY (	(DRY RUDDED	) LBS.?CU.FT.	-		111	

LN 4/18/75

LAWRENCE SANGRAVCO - CONCRETE MIXES F		÷
IX NO.	43	.44
MIX DESIGN:		
(1) Type I Gls. Fls. Cement - lbs.	611	611
(2) Concrete Sand (S.S.D.) - 1bs.	1201	1100
(3) 3/4" Crushed Stone (S.S.D.) - 1bs.	660	675
(4) $1-1/2$ " Crushed Stone (S.S.D.) - 1bs.	1385	1400
(5) Net Water - 1bs.	314	300
(6) Darex $-$ ozs.		
(7) Total Batch Weight - 1bs.	9.0 4171	<u>9.0</u> 4086
IX YIELD - cu. ft.	27.9	27.0
Sand - % of total aggregate by volume	37.6	35.3
and - % of total aggregate by vortime	97.0	57.5
CONCRETE TESTS:	0.511	0.107
(1) Water - 1b per 1b. cement	0.514	0.491
(2) Net gallons water per sack	5.79	5.53
(3) Slump - inches	2.75	2.5
(4) % Entrained Air - Chace	5.3	5.2
(5) % Entrained Air - Pressure	6.0	5.1
(6) Concrete Density - 1bs./cu.ft.	1149.5	151.6
(7) Calculated cement content - sacks per cu.yd.	6.29	6.51
OMPRESSIVE STRENGTH - 6" x 12" cyls PSI:		
Days	2085	_
Days	2195	2475
verage 3 Day Strength	2140	
verage y bay borengar	2140	-
7 Days	2600	
7 Days	2650	2865
Days	2705	3025
Days	210)	5027
verage 7 Day Strength	2650	2945
L4 Days	2900	3200
4 Days		
4 Days	3005	3255
verage 14 Day Strength	2955	3230
verage 14 Day buildingun	2,777	12 10
8 Days	3110	-
28 Days	3200	3395
8 Days	2990	3500
verage 28 Day Strength	3100	3450
Bere		
Sand -10% 275	35	
n Sand +0% 325		
125, 50-50		
4		

## LAWRENCE SANGRAVCO - MORTAR MIXES - MARCH 4, 1975

MIX NO.	- 53	54
MIX BY WEIGHT	1:3	1:3
MATERIALS FOR 1 - SK BATCH: (1) Type I Gls. Fls. Cement - lbs. (2) Concrete Sand (S.S.D.) - lbs. (3) Net Water - lbs. (4) Darex - lbs. (5) NVX Powder - gms.	94 282 51.1 1.38	94 282 51.6 - 4.9
<pre>CONCRETE TESTS:    (1) Water - lbs. per lb. cement    (2) Net gallons water per sack    (3) Slump - Inches    (4) % Entrained Air - Chace    (5) Concrete Density - lbs./cu.ft.    (6) Calculated Cement Content - Sacks per cu. yd</pre>	0.543 6.12 3.25 5.1 134.2 8.48	0.549 6.18 2.75 4.1 137.3 8.67
COMPRESSIVE STRENGTH - 3" x 6" cyls PSI:		
3 Days	2505	2785
7 Days 7 Days	2630 2730	3280 3425
Average 7 Day Strength	2680	3350
28 Days	3665 3935	4175 4300
Average 28 Day Strength	3800	4235

:

## LAWRENCE SANGRAVCO SAND - MARCH 5, 1975

SIZE	% RETAINED	CUMULATIVE % RETAINED	% PASSING	
3/8"	0.0	0.0	100.0	
No. 4	0.7	0.7	99.3	
8	14.2	14.9	85.1	
16	21.8	36.7	63.3	
30	23.7	60.4	39.6	
50	19.6	80.0	20.0	
100	10.6	90.6	9.4	
TOTALS	90.6	283.3	-	

2.83
CLEAR
5.18
0.46

LENS FALLS PORTLAND CEMENT COMPANY IVISION OF THE FLINTKOTE COMPANY

LAWRENCE SANGRAVCO - CONCRETE MIXES - MARCH 6, 1975					
MIX NO.	55	56	57	58	
<pre>MIX DESIGNS: (1) Type I Gls. Fls. Cement - lbs. (2) Concrete Sand (S.S.D.) - lbs. (3) 3/4" Crushed Stone (S.S.D.) lbs. (4) 1-1/2" Crushed Stone (S.S.D.) lbs. (5) Net Water - lbs. (6) Darex - lbs. (7) 25% Vinsol Resin - oz. (8) Total Batch Weight - lbs.</pre>	$ \begin{array}{r} 611\\ 1201\\ 660\\ 1385\\ 315\\ 9\\ -\\ 4172 \end{array} $	$ \begin{array}{r} 611\\ 1201\\ .660\\ 1385\\ 315\\ 9\\ -\\ 4172 \end{array} $	$ \begin{array}{r} 611\\ 1201\\ 660\\ 1385\\ 309\\ -\\ 4.9\\ 4166\\ \end{array} $	$ \begin{array}{r} 611\\ 1201\\ 660\\ 1385\\ 315\\ -\\ 2.45\\ 4172 \end{array} $	
MIX YIELD - cu. ft. SAND - % of total aggr. by vol.	27.1 27.1 28.3 27.1 ggr. by vol. 37.6 37.6 37.6 37.6				
<pre>CONCRETE TESTS:   (1) Water - lb. per lb. cement   (2) Net gallons water per sack   (3) Slump - Inches (approx.)   (4) % Entrained Air (Chace)   (5) Concrete Density - lbs./cu.ft.   (6) Calculated Cement Content     Sacks per cu. yd.</pre>	0.515 5.80 2.5 4.4 154.0 6.48	0.515 5.80 2.5 4.3 153.8 6.47	0.506 5.70 2.5 6.5 147.1 6.20	0.515 5.80 2.5 5.0 154.0 6.48	
COMPRESSIVE STRENGTH - 4" x 8" cyls. PSI:	÷ .				
1 Day	1895	1880	1585	-	
7 Days 7 Days	3055 3245	3080 3225 3150	2540 2740	3110 3265	
Average 7 Day Strength	31.50		2640	3185	
14 Days 14 Days	3110 3470	3445 3455	2835 2960	3245 3725	
Average 14 Day Strength	3290	3450	2895	3485	
28 Days 28 Days	3340 3945	3535 3630	32 30 3350	3945 4035	
Average 28 Day Strength	3645	3580	3290	3990	

NOTES: (1) In mix No. 55 Darex added following addition of aggregates, cement 1/2 of water

and some mixing.
(2) In mix No. 56 Darex added with coarse aggregate and 1/2 of water. Mixer then started and remainder of materials added.

ln 4/18/75 DIVISION OF THE FLINTKOTE COMPANY

MIX NO.	60	61	
AGGREGATE IDENTIFICATION:	Lab Washed	As Received	
MIX DESIN:			
(1) Type I Gls. Fls. Cement - 1bs.	611	611	
(2) Concrete Sand (S.S.D.) - 1bs.	1201	1201	
(3) 3/4" Crushed Stone - 1bs.	660	660	
(4) $1-1/2$ " Crushed Stone - 1bs.	1385	1385	
(5) Net Water - 1bs.	285	310	
(6) Darex - ozs.	9	9.75	
(7) Total Batch Weight - 1bs	4142	4107	
MIX YIELD - CU. FT.	26.7	27.3	
Sand - % of Total Aggr. By volume	37.6	37.6	
CONCRETE TESTS:			
(1) Water - 1b. per 1b. cement	0.466	0.507	
(2) Net gallons water per sack	5.25	5.71	
(3) Slump - Inches (Approx.)	2.5	2.5	
(4) % Entrained Air (Chace)	4.8	4.6	
(5) Concrete Density - 1bs./cu. ft.	155.0	152.4	
(6) Bleeding	NONE	LIGHT	
(7) Calculated cement content - sacks per cu.		6.42	
COMPRESSIVE STRENGTH - 4" x 8" cyls PSI:			
1 Day	2030	1765	
7 Days	3645	3200	
7 Days	3660	3310	
ישעיט	2000	0100	
Average 7 Day Strength	3650	3255	
14 Days	3890	3380	
14 Days	4100	3565	
Average 14 Day Strength	3995	3475	
28 Days	4355	3765	
28 Days	4610	3835	
Average 28 Day Strength	4480	3800	

LAWRENCE SANGRAVCO - CONCRETE MIXES - MARCH 12, 1975