

INVESTIGATION OF RETARDING ADMIXTURES FOR
PORTLAND CEMENT CONCRETE

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STATE OF VERMONT
DEPARTMENT OF HIGHWAYS
MATERIALS DIVISION

INVESTIGATION OF RETARDING ADMIXTURES FOR
PORTLAND CEMENT CONCRETE

INTERIM REPORT NO. I

BY

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RETARDING ADMIXTURE INVESTIGATION

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A. INTRODUCTION

One characteristic of concrete that makes it especially valuable as a construction material is its ability to remain plastic for a period of time before it stiffens and starts to develop strength. This delay in set provides time for the constructor to place, consolidate, and finish the concrete before it hardens.

The length of time that concrete remains plastic varies from a few minutes to many hours, depending on the type and brand of cement used and the temperature and weather conditions prevailing at the time the concrete is placed. High temperatures reduce the setting times considerably and often on hot days there will be insufficient time to properly place concrete on certain types of construction.

One type of construction that often requires more retardation of set is bridge decks. If the concrete above a pier hardens before all of the dead load deflection has developed in the deck, more and more stress will be applied to the hardened concrete as the placing continues until the concrete over the pier fails in flexure and a crack is formed. One solution to this problem is to divide the bridge deck into sections and place the areas immediately above the piers last. In this manner the dead load deflection will have developed in the deck before the concrete over the piers has hardened. However, this method of construction requires the use of a considerable amount of additional labor and the installation of numerous headers, which often results in a rough riding surface.

Another solution to the problem would be to produce concrete that would not stiffen and get hard until all sections of the deck had been placed. In this manner placing of the concrete could begin at one end of a deck and continue uninterrupted to the other end. This method allows for the efficient use of mechanical finishing machines - thus providing a smoother riding surface.

The factors affecting the setting time of concrete have been discussed in many reports but none deal with the combinations of admixtures and local materials used in Vermont Highway construction.

The purpose of this investigation is to examine through testing all retarding admixtures currently being used on State of Vermont highway projects.

This series of laboratory tests were performed in accordance with AASHO M 194 with some modifications due to limited facilities and equipment.

B. MATERIALS

In conducting this investigation, it was intended to test the retarding admixtures in combination with the cement and aggregate used in actual field conditions. Most of the materials used for this report were obtained from local producers.

(a) Aggregates:

The aggregate sources were chosen due to their geographical location as well as to their geological composition. For this reason, six locations were sampled representing a good cross-section of the aggregates produced for Vermont consumption. All aggregates were tested for gradation and mineral composition. Tests were conducted concerning thin and elongated piece count and fractured face count of coarse aggregates, as well as organic impurities in sand. The results of these tests are recorded in the individual Retarder Test Reports in the Appendix.

Following is listed the aggregate sources used:

Lebanon Crushed Stone	-	W. Lebanon, N. H.	Test Report	A
* J. P. Carrara & Sons	-	N. Clarendon, Vt.	"	" B
* Calkins Construction	-	Coventry, Vt.	"	" C
Lucien Demers - Winooski & S. T. Griswold	-	Williston	"	" D
* W. E. Dailey & Sons	-	South Shaftsbury, Vt.	"	" E ₁ &E ₂
Kelley Construction	-	Websterville, Vt.	"	" F

*Crushed gravel

(b) Cement:

All of the cement currently used in the Vermont Highway program is supplied by three producers. It is continually being tested by the Highway Materials Laboratory for its physical properties which must conform to AASHO M 85 and/or M 13⁴. Since the Department allows both Type I and Type II cements, it was decided that both Types should be included in this program. The individual Retarder Test Reports indicate the brand and type of cement used in conjunction with each aggregate source. Most of the cement in these tests was sampled directly from the silos of the various ready-mix producers. Following is listed the cement manufacturers:

Glens Falls Portland Cement Co.	-	Glens Falls, N. Y.
Universal Atlas Cement Co.	-	Hudson, N. Y.
North East Cement Co. Inc.	-	Syracuse, N. Y.

(c) Retarders:

The retarders investigated in this program represent those brands currently used in Vermont. They represent three general chemical types: lignosulfonic acids, hydroxylated carboxylic acids, and carbohydrates. No effort was made to perform chemical tests upon these products. Most samples were obtained directly from the ready-mix producers stock. Again, they were used in the same combinations as in actual field application.

Following is listed the suppliers and the brand names:

Grace Construction Materials	-	Cambridge, Mass.	Daratard & Daratard H.C.
Sika Chemical Corporation	-	Passaic, N. J.	Plastiment
Masterbuilders	-	Cleveland, Ohio	Pozzolith 100 KR & MBUC

(d) Air Entraining Admixtures:

The use of non-air entrained cement necessitated the addition of an admixture to obtain the desired air content. Vermont Specifications require an

air content to range from 5% to 8% of the concrete volume. Several commercial products are available but the brand most commonly used by Vermont ready-mix producers is Grace Chemicals' "Darex". This was used in all test mixtures with but one exception. Master Builders Vinsol Resin was used in conjunction with their retarders since this practice prevails in the field.

C. PROCEDURES

The procedures used in mixing throughout this investigation followed as closely as possible that which is required by AASHO T 126 and every effort was made in controlling uniformity of detail.

These tests were performed using Vermont's Standard Class AA ($6\frac{1}{2}$ bags/cy) concrete. Three batches of retarded concrete were compared with three batches of reference or non-retarded concrete for each test as required in AASHO M 194, et. al. Seven tests were performed in order to best represent the majority of combinations common throughout Vermont.

The ingredients were combined in the sequence prescribed in AASHO T 126 with adjustments being made for a slump of 2 to 3 inches. All concrete was blended in a $1\frac{1}{2}$ cu. ft. Sears Roebuck Revolving Drum Type Mixer.

After the concrete was removed from the mixer, the following tests were performed:

1. Slump of Portland Cement Concrete (AASHO T 119).
2. Air content of freshly mixed concrete by Chase Indicator (AASHO T 199).
3. Air content of freshly mixed concrete by Pressure Method (AASHO T 152).
4. Weight per cubic foot & yield of concrete (AASHO T 121).
5. Time of setting of concrete mixtures by penetration resistance (AASHO T 197).
6. Making & curing concrete compression test specimens in the Laboratory (AASHO T 126).

The air content was tested using the Chase Air Indicator with occasional checks using the Pressure Meter Method. When the latter was used, the weight per cu. ft. and yield were also determined.

Penetration tests were performed in accordance with AASHO T 197 using an "Acme" Penetrometer. A six inch cube of mortar was screened from each batch. A mechanical sieve shaker and standard eight inch #4 sieves were used to remove the mortar from the concrete. The steel molds were covered upon being filled in order to keep the mortar from surface drying prematurely. Temperature of the mixes and the laboratory facilities were maintained at $73.4^{\circ}\text{F} \pm 3^{\circ}\text{F}$.

Due to a limited supply of standard laboratory steel cylinder molds, it was felt that it would be possible to make a study comparing the compressive strength of cylinders cast in various types of commercial molds. To this end, steel, plastic, and paraffined cardboard molds were utilized. Three 6" x 12" cylinders were cast from each batch of concrete. These cylinders were broken at ages of three, seven, and twenty-eight days.

Freeze-thaw tests comparing the durability of retarded and non-retarded concretes were run in cooperation with the State of New Hampshire. Space was shared in their testing cabinet resulting in a limited number of specimens to be compared. Thus, only two aggregate sources were chosen - one being crushed rock, the other being crushed gravel. These tests were made in addition to the before mentioned tests and in accordance with AASHO T 161 - Resistance of Concrete Specimens to Rapid Freezing and Thawing in Water. Specimens were cast in 3" x 2 3/4" x 15" steel molds, then cured for fourteen days prior to being shipped to New Hampshire.

A note of explanation from New Hampshire:

Four groups of 3 beams each were received from Vermont to be placed in New Hampshire's freeze-thaw cabinet. All the samples were weighed and a sonic fundamental transverse frequency was determined on each. Two beams from each set were placed in the freeze-thaw cabinet and the third one was placed in the moisture room to compare with the others after 300 cycles were completed. After every 50 cycles or so the samples were removed from the cabinet and reweighed and a new frequency measured.

After 300 cycles the samples were broken in midpoint flexure and the two remaining halves were capped and broken for compressive strength.

The concrete mix design and all results may be found in "Discussion of Tests" (f. Durability).

D. DISCUSSION OF TESTS

In conducting these tests, it was sought to determine if the retarding admixtures as used in actual practice conformed to the Vermont Department of Highways specifications. Therefore, each series of batches comparing the retarded concrete and its reference concrete are separate tests and the comparison of one brand of retarder to that of another should be avoided. Although the graphs, etc. show the collective results of the complete series, each test should be considered singularly.

(a) Consistency:

Although every effort was made to maintain a slump of 2" to 3", there was an occasional batch slightly less than two inches. No attempt was made to readjust those mixtures once the slump test was performed. However, no mixtures were allowed having a slump in excess of three inches. It was found that the minor variations within this range had a negligible effect on the setting time of any of the concrete. The stiffer mixtures proved to be much more difficult to screen for the penetration test. The results of the slump tests for each batch are listed in the Appendix.

(b) Water Content:

The addition of the retarder caused wide variations in water content from that of the reference concrete. With one exception, the retarders decreased the amount of water necessary to produce the desired consistency. In test reports E_1 and E_2 (See Appendix) the change in water content was negligible. This was possibly caused by the cement which displayed the properties of "false set" in the reference mixes. Further tests regarding these retarders should be conducted prior to forming definite conclusions.

Claims of the various manufacturers about their products water reducing capabilities were, in some cases, exaggerated but most met the minimum requirements. The lignosulfonate (Daratard) proved to be the most capable water reducer of all the chemical types.

The minimum water reduction caused by retarders is specified to be 5%. The before mentioned tests (E_1 & E_2) failed to meet this requirement and Test Report "C" also failed by 0.4%. It should be mentioned that under actual field conditions, the percentage of water reduction will vary with changes in temperature. According to a report submitted by the State of Nebraska, the mixing water requirements of all retarders was increased at 100°F.

No attempt was made to measure the amount of bleeding water from the penetration test specimens. It was noted that most retarded mixes had slightly more bleeding than the reference mix.

Your attention is directed to Figure I which shows the effect of retarders on mixing water requirements.

(c) Air Entrainment:

There are many factors that effect the air content of Portland Cement Concrete. Among them may be the difference in cement fineness, temperature, water content, and aggregates. The addition of retarders also effects the air volume depending on its interaction with the other ingredients. In these tests, all retarders tended to entrain varying amounts of air. Thus, the air entraining agents had to be adjusted to obtain the proper air contents within the limits desired. The lignosulfonic retarder entrained considerably more air than did the other types. Several comparisons were made between the Chase Air Indicator and the Pressure Meter method of testing entrained air. With one exception, all results compared within 0.5%, which helps to confirm the reliability of the Vermont modified version of the Chase Air Indicator.

(d) Retardation:

The retarding admixtures were added in the quantity recommended to cause an increase from 50 to 60 percent in the setting time of the retarded concrete as compared to the setting time of the reference concrete. This requirement is currently specified by the State of Vermont. However, AASHO specifications do not include this requirement but specify instead a maximum increase in setting time of three hours. The two specifications are, in many instances, not compatible.

Furthermore, none of the tests in this program met both specifications. No attempt was made to rerun any test using an adjusted quantity of retarder. The reason for accepting the obtained results was due to the difficulty in acquiring the necessary amount of concrete ingredients, as well as the fact that each brand of cement and type of aggregate affected the setting time of the mixes differently. This was proven in the reference mixes as well as those mixes using the same retarder. Your attention is directed to Figure 2-A (Time of Set of Concrete Mixtures).

The initial set (500 psi - penetration resistance) is the standard measure showing the hardening rate of concrete, but it should be noted that this point is well past the time when finishing or working of the mix can be easily performed. Figure 2-B shows the curves of both the reference and retarder mixes as plotted from the results of the Penetration Resistance Tests.

(e) Compressive Strengths:

Vermont Specifications require a compressive strength of one-hundred and ten percent when using a retarded mixture relative to that of its reference mixture. This specification refers to all retarders whether they are classed as water-reducing retarders or not. AASHO requires this increase in strength for water-reducing retarders only, while those classed simply as retarders are required to have a relative strength of ninety percent. As shown in Figure 3-F, an increase in compressive

strength due to retardation was realized in every test, although some did not surpass the Vermont minimum requirement at all test ages.

The many factors affecting compressive strengths of cylinders include variations in air content, cement content, water-cement ratios, handling, curing, and even the materials from which the molds are constructed. While increased air entrainment usually reduces strength, it was observed, as previously noted, that retarding admixtures generally increased strength. Also, the steel and plastic molds generally produce higher strength cylinders than those cast in paraffin treated cardboard molds. Exceptions were noted at all ages and are indicated in Figure 3-A. Your attention is directed to Figures 3-B through 3-E, which graphically show the comparative cylinder strengths relative to the type of molds. Note that this comparison was performed on three and seven day specimens only due to limited equipment and higher early sensitivity. The twenty-eight day tests were all cast in treated cardboard molds.

Conclusive proof of the desirability of one mold in preference to another is difficult to establish from this series of tests. A factor such as a three percent variation in air content may have profound influence on strength.

(f) Durability:

Vermont and AASHO specifications require that a minimum Relative Durability Factor of 80 must be obtained when comparing a retarded concrete to a reference concrete through Freeze-thaw testing. The results obtained show that all samples tested were well above this minimum.

All information concerning properties of the concrete mixtures used in freeze-thaw testing are shown in Figure 4A

The results obtained from New Hampshire were calculated according to AASHO T-161 which determined the Relative Durability Factor. As indicated in Figures 4B and 4C, the resulting Factor was 100.6 for batch 1 as compared to 2 and 97.3 for batch 4 as compared to 3. This illustrates that the retarders used have no significant effect upon the durability of these mixes.

The samples which were subjected to freeze-thaw testing completed 300 cycles with relatively little weight loss and no physical damage was detectable.

The samples were all physically broken in flexure and then in compression. Results of the actual compressive tests are all less than those computed from the sonic testing.

Figure 4D shows the results of all strength tests performed by the New Hampshire Materials Division. For further details concerning these tests, the complete report is available in the files of the Materials Division.

E. SUMMARY & CONCLUSIONS

1. Strength of cylinders increased in all cases when a retarding admixture was introduced. This increase did not always meet minimum specifications.
2. The addition of retarding admixtures caused no significant adverse effect on the durability of concrete.
3. Retarding admixtures tested, in most cases, reduced the water content of the concrete for any given consistency. The one exception to this statement was possibly caused by false set of cement - not the retarder.
4. Strength of cylinders varied considerably when comparisons were made using steel, plastic, and cardboard molds. With few exceptions, the steel and plastic molds produced the highest strengths while cylinders made in cardboard molds usually broke lower.

5. The retarders in many cases did not meet the requirements specified due to the use of too many variables.
6. Retarder dosages recommended by their manufacturers resulted in higher quantities than necessary when applied under laboratory conditions.
7. Variable amounts of retardation may be expected with various combinations of materials.
8. Minor variations in consistency had a negligible effect on the setting time of the concrete.
9. The amount of air-entraining agent required in the retarded concretes was dependent upon each retarder and the combination of materials that is was used with. Lignosulfonate retarders increased the air content most noticeably.
10. Strength of cylinders varied inversely with the air content of the concrete. As the air-entrainment increased, strength of cylinders decreased.

F. RECOMMENDATIONS

1. The 1964 Vermont Standard Specifications should be revised to conform with AASHO Specifications in cases where there is conflict or disagreement.
2. For acceptance testing, samples of the product to be tested should be obtained from the Supplier as opposed to sampling from Ready-Mix inventories.
3. The testing of any one ingredient in a concrete mixture should be the only variable; all other materials should remain constant.

4. All cylinders should be cast in molds of similar materials in order to minimize their influence on compressive strength.
5. The difference between air contents of both the reference concrete and the retarded concrete should be controlled within 0.5%.
6. The rate of addition of retarding admixture for field conditions should be established through experience and actual field testing.

FIGURES

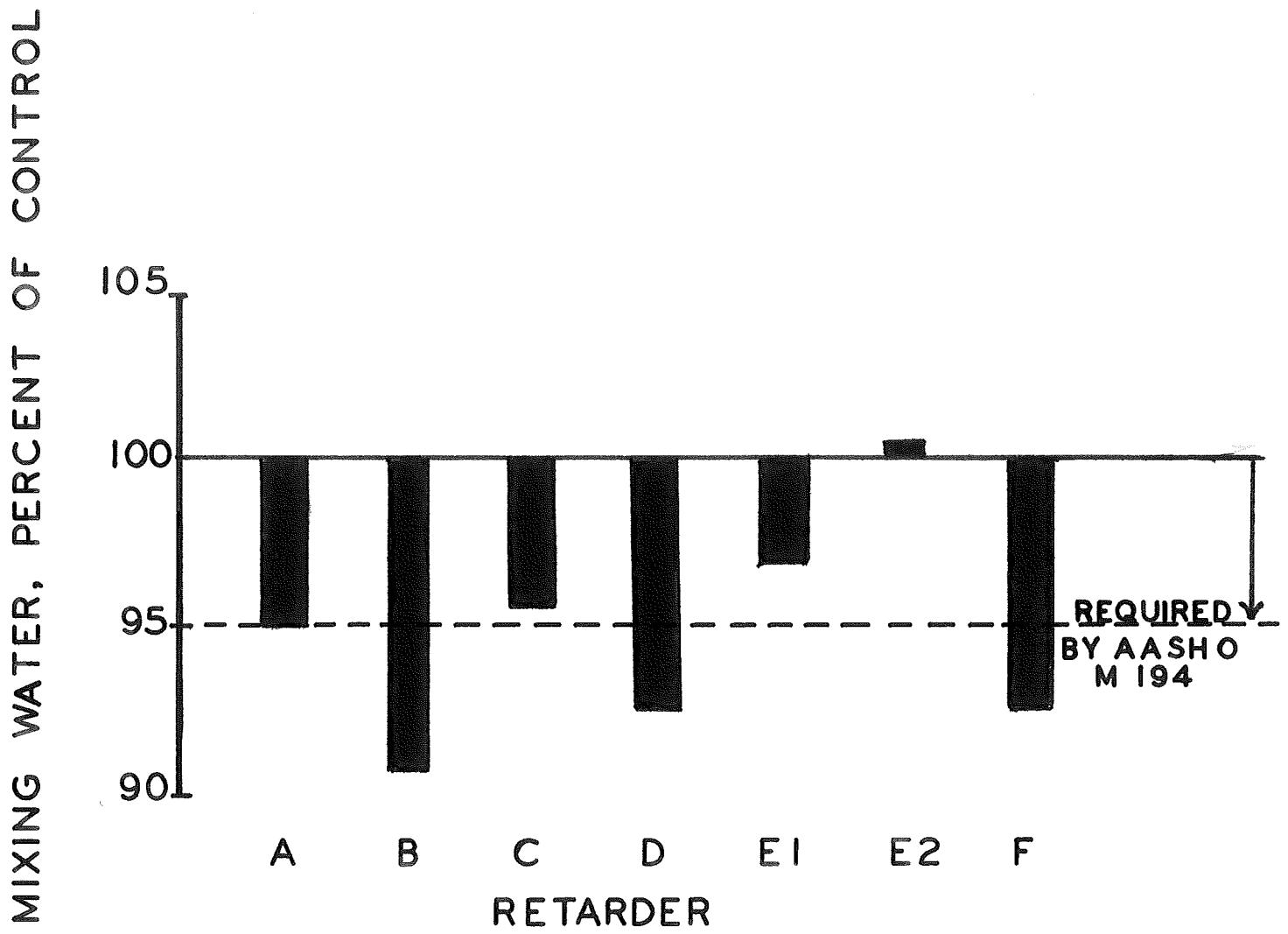


FIGURE I WATER CONTENT
AVERAGE PERCENTAGE OF VARIATION DUE TO RETARDER

INITIAL SET 500 PSI

TEST	FINAL SET 4000 PSI		
	REF	RET	PERCENT INCREASE
	TIME IN HOURS	2 INCREASE IN HOURS	TIME IN HOURS
A	4.63	7.69	66
B	6.08	9.39	54
C	6.31	8.98	42
D	5.37	7.77	45
EI	5.67	8.92	57
E2	5.67	10.26	81
F	4.32	8.27	91

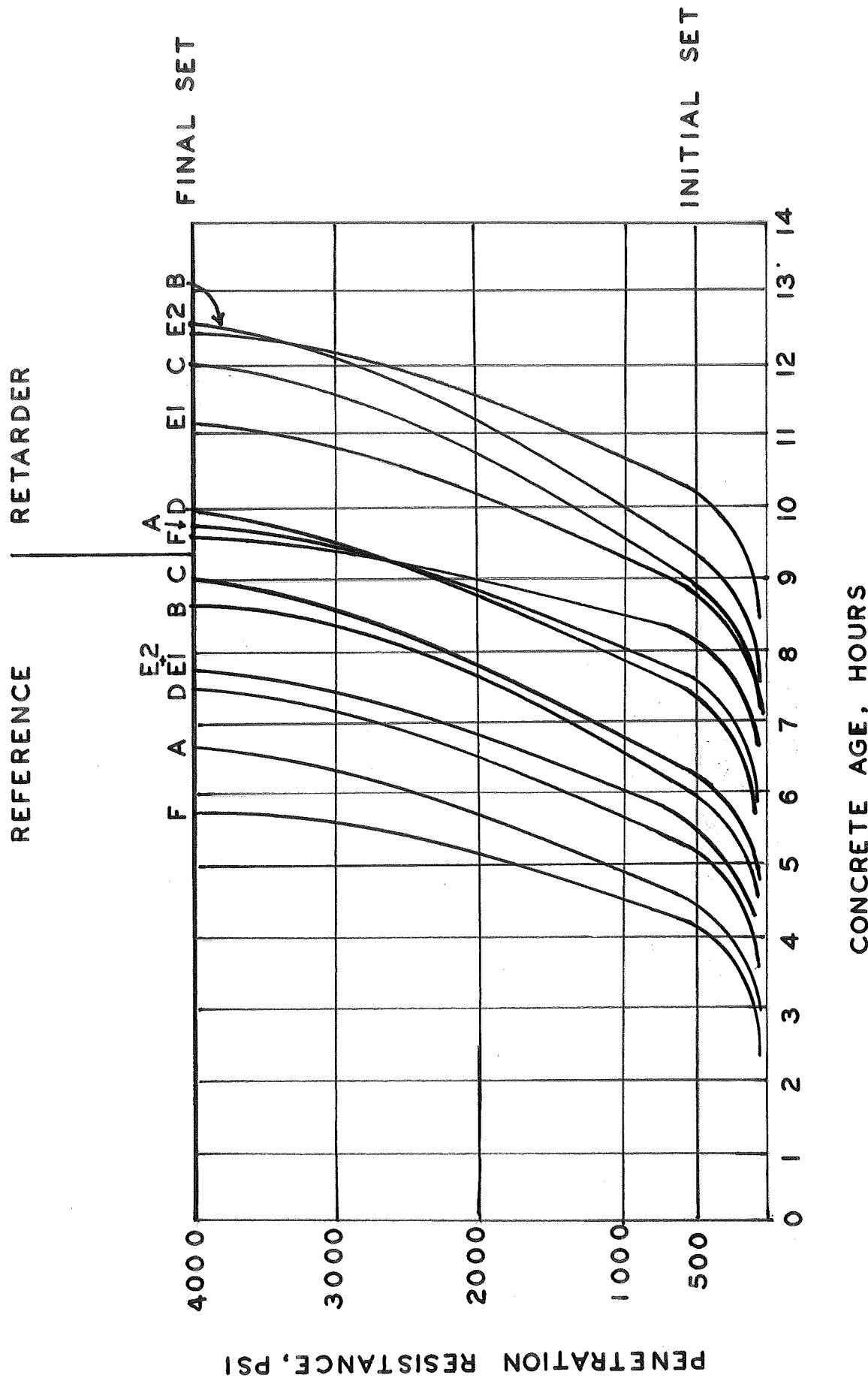
1. 1964 VERMONT HIGHWAY SPECIFICATIONS
 2. AASHO M 194

FIGURE 2A. TIME OF SET OF CONCRETE MIXTURES

FIGURE 2B TIME OF SET CURVES REFERENCE MIX VS. RETARDER MIX

FIGURE 2B TIME OF SET CURVES REFERENCE MIX VS. RETARDER MIX

(14)



COMPRESSIVE STRENGTHS OF CYLINDERS

FIG 3A

(15)

Retarded Concrete Test No.	3 DAY			7 DAY			28 DAY		
	Steel	Plastic	Cardboard	Steel	Cardboard	Cardboard	Cardboard	Cardboard	Cardboard
A 4041	3855	3767	4616	4403	4244	4297	5535	5429	
B 4253	2467	3387	4297	3289	4183	4554	3767	4793	
C 3218	3272	2511	4149	3581	3431	4757	4185	5173	
D 3440	4899	3749	4138	5217	4863	4227	6048	5394	
E1 4740	4439	3961	4757	4722	4704	5474	5801	5341	
E2 4191	4014	3643	5305	4536	4459	5474	5456	5252	
F 2759	3395	3245	5210	5422	3723	3599	3979	4695	
Reference Concrete									
A 2935	3280	2582	3519	3445	3210	4527	4439	4174	
B 2953	3334	2785	3793	3599	3652	4403	4793	4359	
C 2502	2520	2087	3749	2829	2989	4793	4563	4545	
D 2502	3749	2935	4094	3988	3272	4200	4342	4147	
E1 4103	3837	3139	4881	3926	3564	4032	5235	4386	
E2 3192	3148	2440	3820	2918	2661	3882	3882	3564	

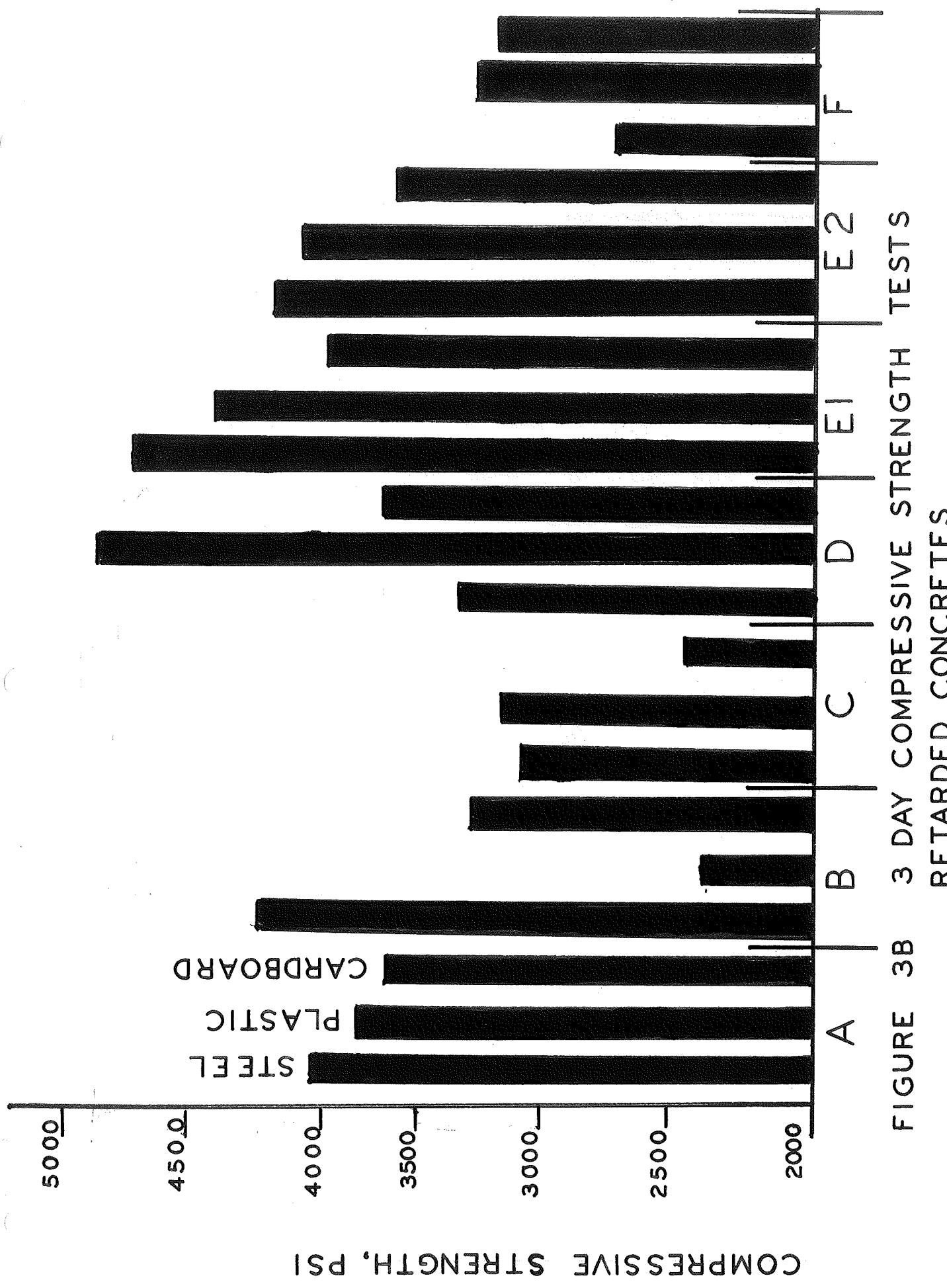


FIGURE 3B 3 DAY COMPRESSIVE STRENGTH TESTS
RETARDED CONCRETES

FIGURE 3C 3 DAY COMPRESSIVE STRENGTH TESTS
REFERENCE CONCRETES

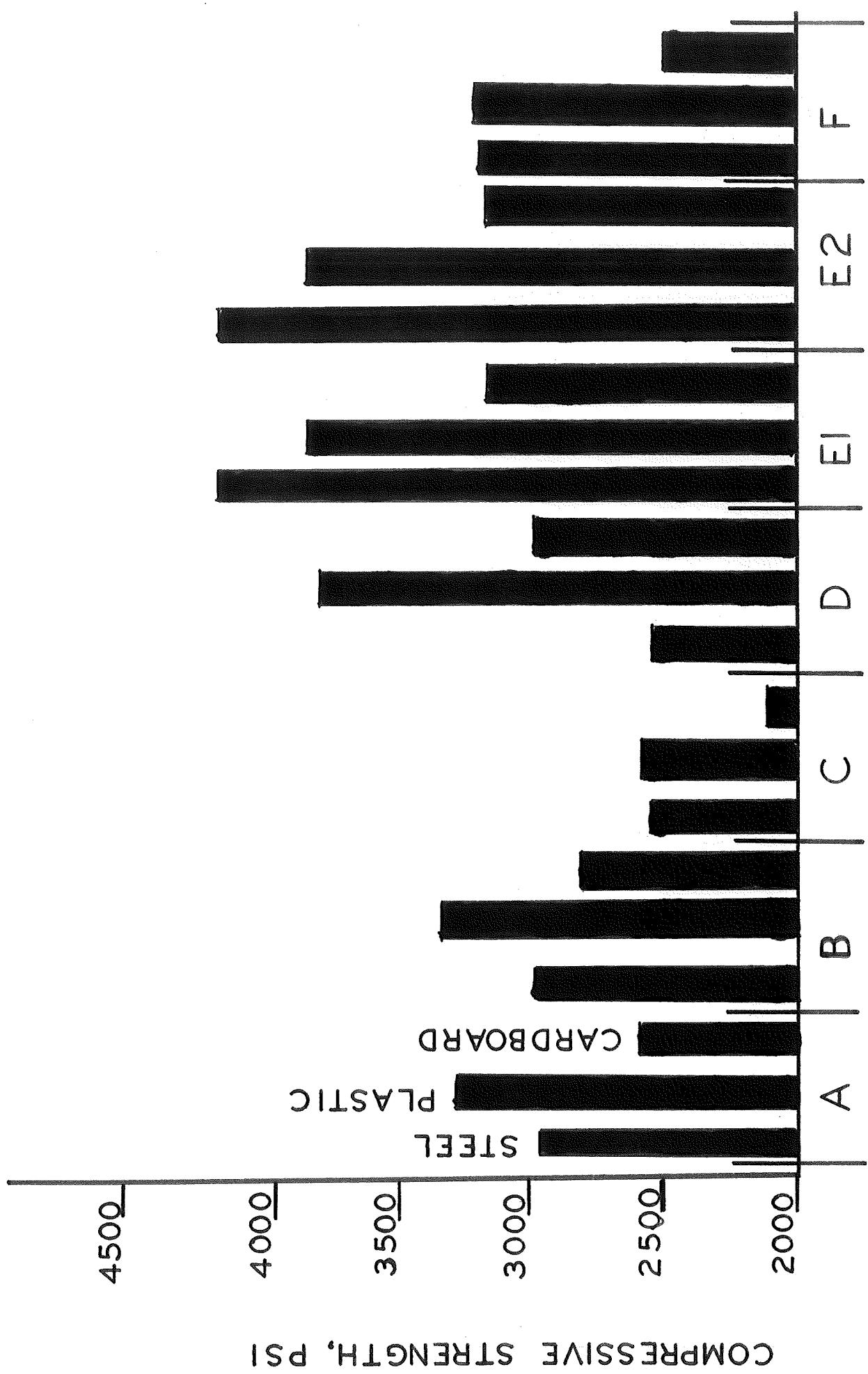
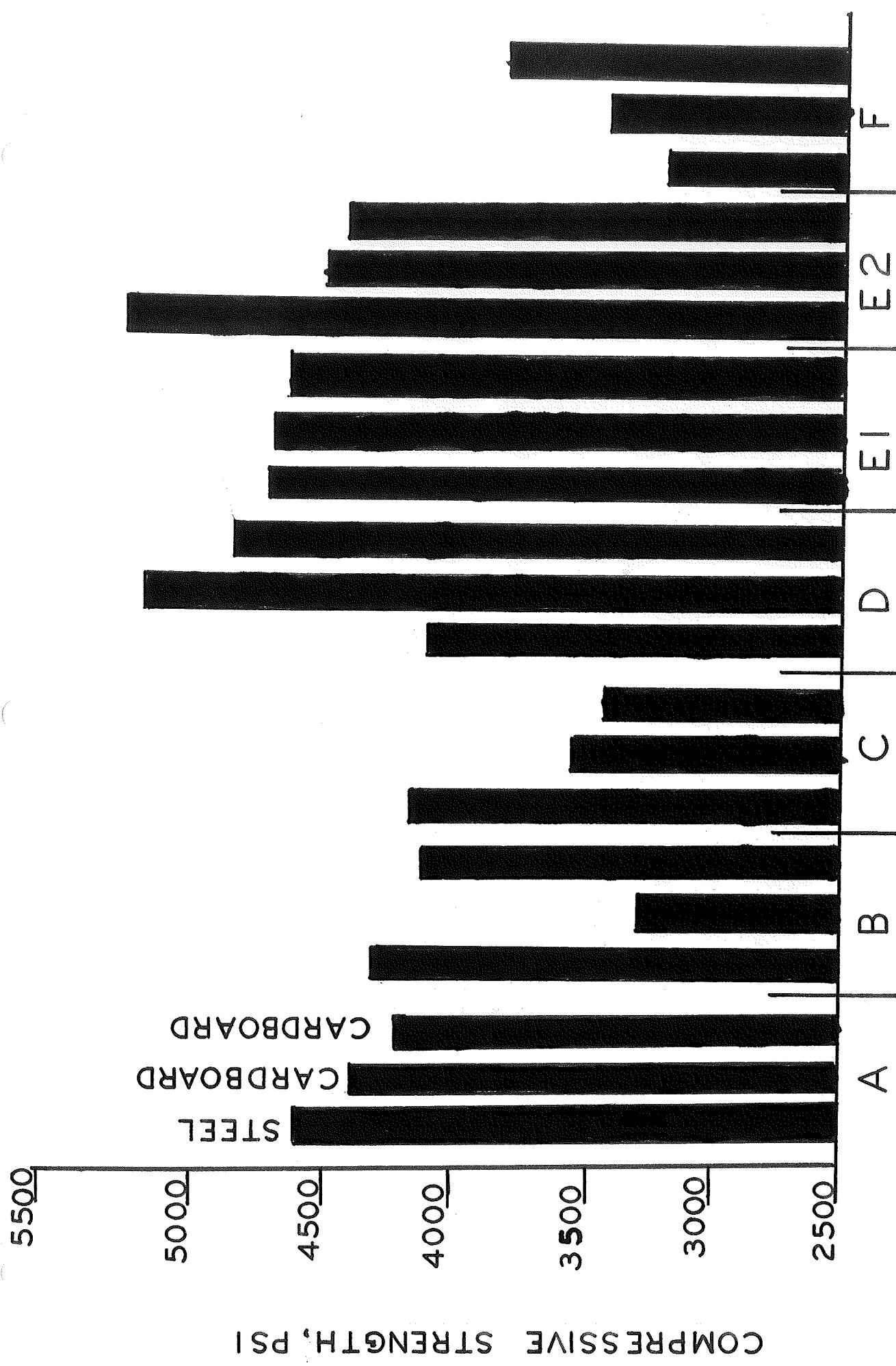
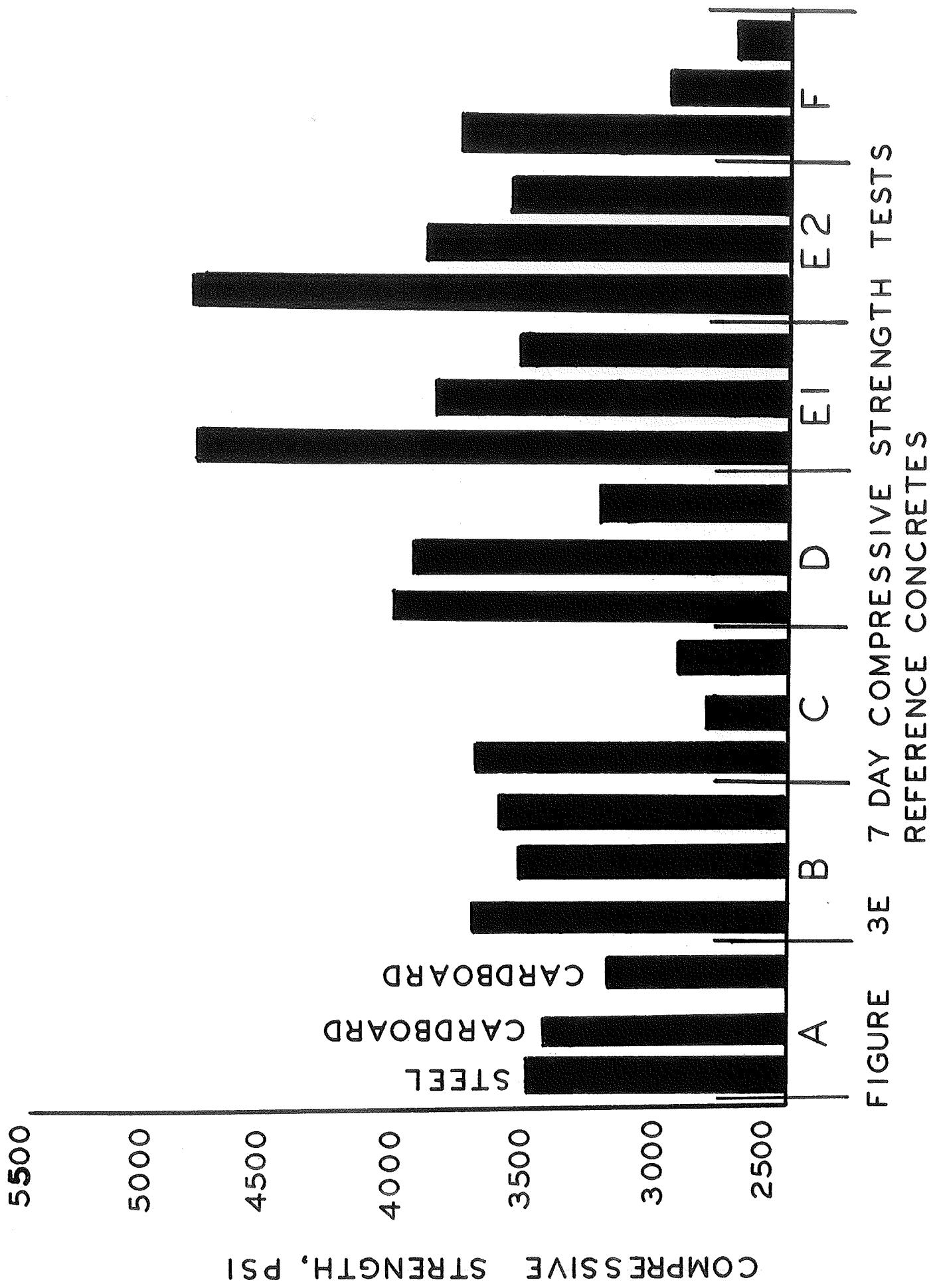
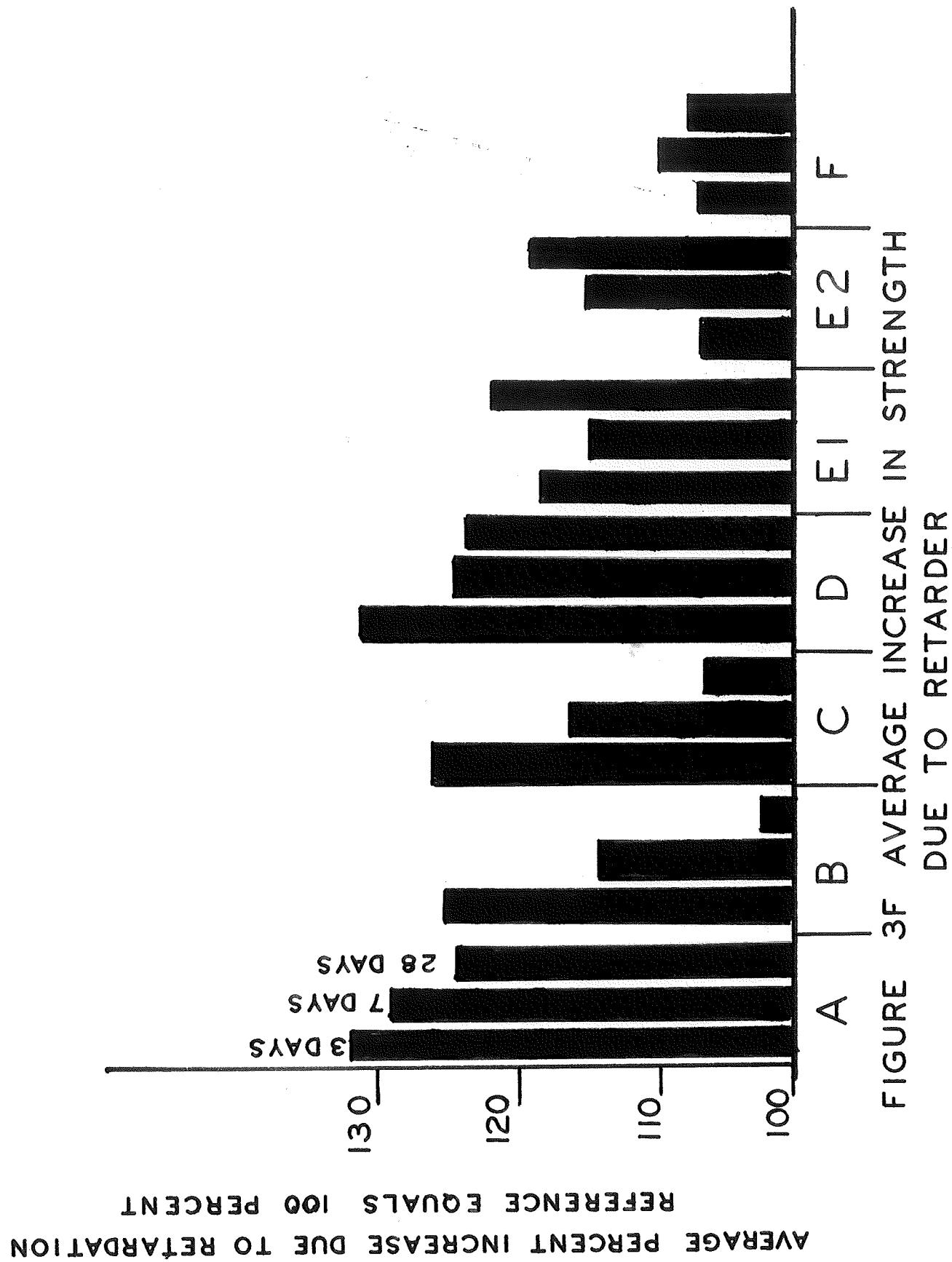


FIGURE 3D 7 DAY COMPRESSIVE STRENGTH TESTS
RETARDED CONCRETES







These beams were made for the purpose of comparing the durability of retarded concrete with that of non-retarded concrete. Two aggregate sources are used incorporating crushed stone and crushed gravel. Two batches for each aggregate were made using proportions as mixed in the field. Different sources of Type I cement were used for each set of batches. A set of three beams were made for each batch. Batches 1 & 2 are from North Clarendon, Vermont, and are crushed gravel. Batches 3 & 4 are from West Lebanon, New Hampshire, and are crushed stone. All batches are based on our standard mix of 6 1/2 bags/cy. The retarder was added to batches one and four.

Following is a list of information concerning each batch:

	<u>Batch #1</u>	<u>Batch #2</u>	<u>Batch #3</u>	<u>Batch #4</u>
<u>Date</u>	2/6/70	2/9/70	2/10/70	2/11/70
<u>-3/4 Stone</u>	43 lb.-7 oz.	43 lb.-7 oz.	44 lb.-4 oz.	44 lb.-4 oz.
<u>Sand</u>	30 lb.-1 oz.	30 lb.-1 oz.	31 lb.-14 oz.	31 lb.-14 oz.
<u>Cement Brand & Type</u>	Iron Clad Type I	Iron Clad Type I	North East Type I	North East Type I
<u>Cement Quant.</u>	15 lb.-5 oz.	15 lb.-5 oz.	15 lb.-5 oz.	15 lb.-5 oz.
<u>Water (design)</u>	7 lb.-2 oz.	7 lb.-2 oz.	7 lb.-2 oz.	7 lb.-2 oz.
<u>Water (actual)</u>	7 lb.-6 oz.	7 lb.-8 oz.	6 lb.-11 oz.	6 lb.-5 oz.
<u>Darex (Air)</u>	4 grams	5 grams	5 grams	5 grams
<u>Retarder</u>	41.5 ml. (Daratard-9 oz./sack)	None	None	11.5 ml. (Daratard "HC" 2½ oz./sack)
<u>Slump</u>	2 1/4"	1 3/4"	2"	2 1/4"
<u>Air</u>	5%	4 1/2%	5 1/2%	5%
<u>Curing</u>	14 days	14 days	14 days	14 days

Design Dry Weights Per Cubic Yard

	<u>3/4 Stone</u>	<u>Sand</u>	<u>Cement</u>	<u>H₂O</u>
N. Clarendon, Vt.	1728	1196	611	285
W. Lebanon, N. H.	1770	1273	611	285

FIGURE 4A FREEZE THAW TEST INFORMATION

VERMONT MARCH 2, 1970

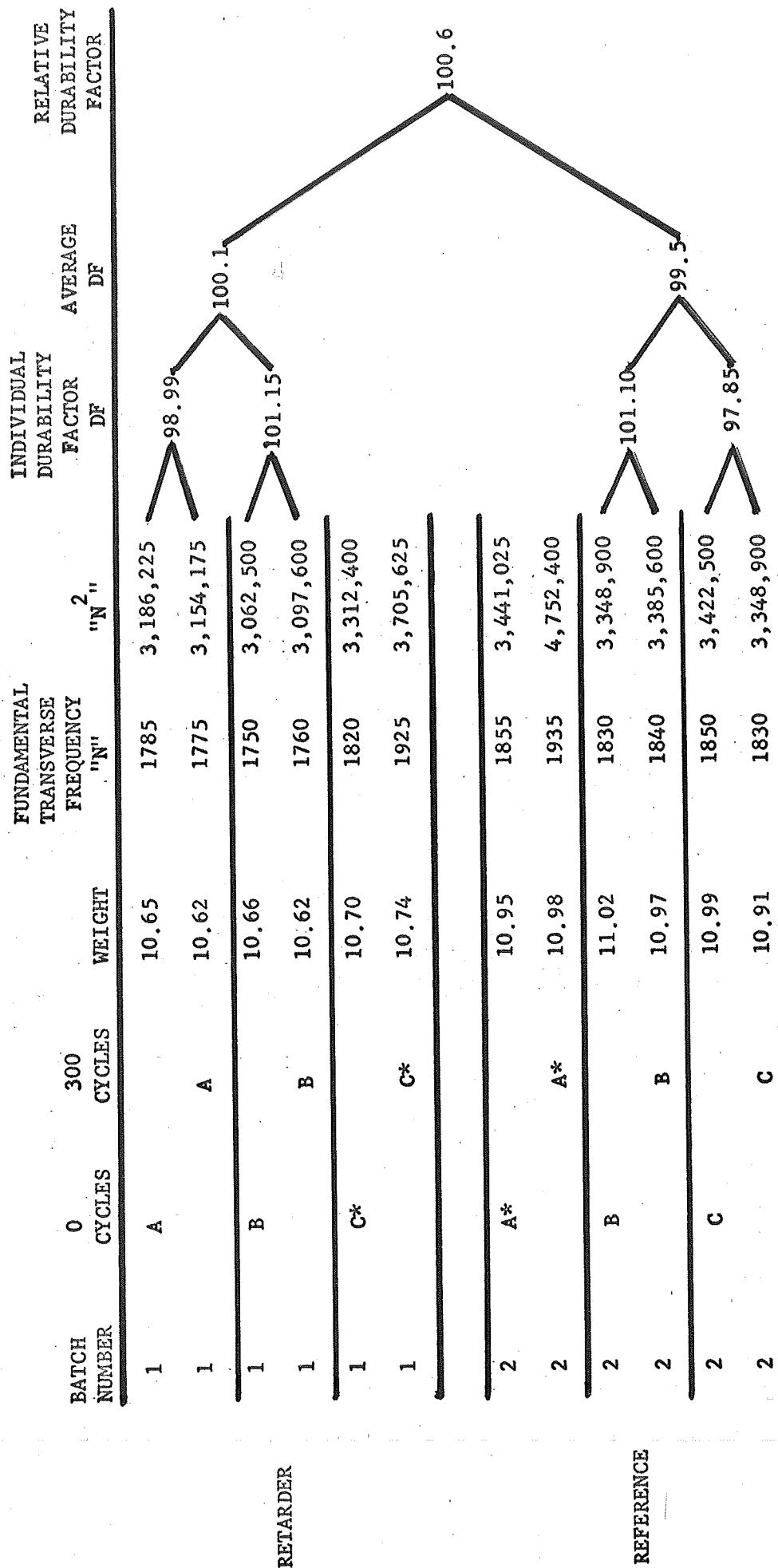


FIGURE 4B RELATIVE DURABILITY FACTOR - FREEZE THAW SPECIMENS

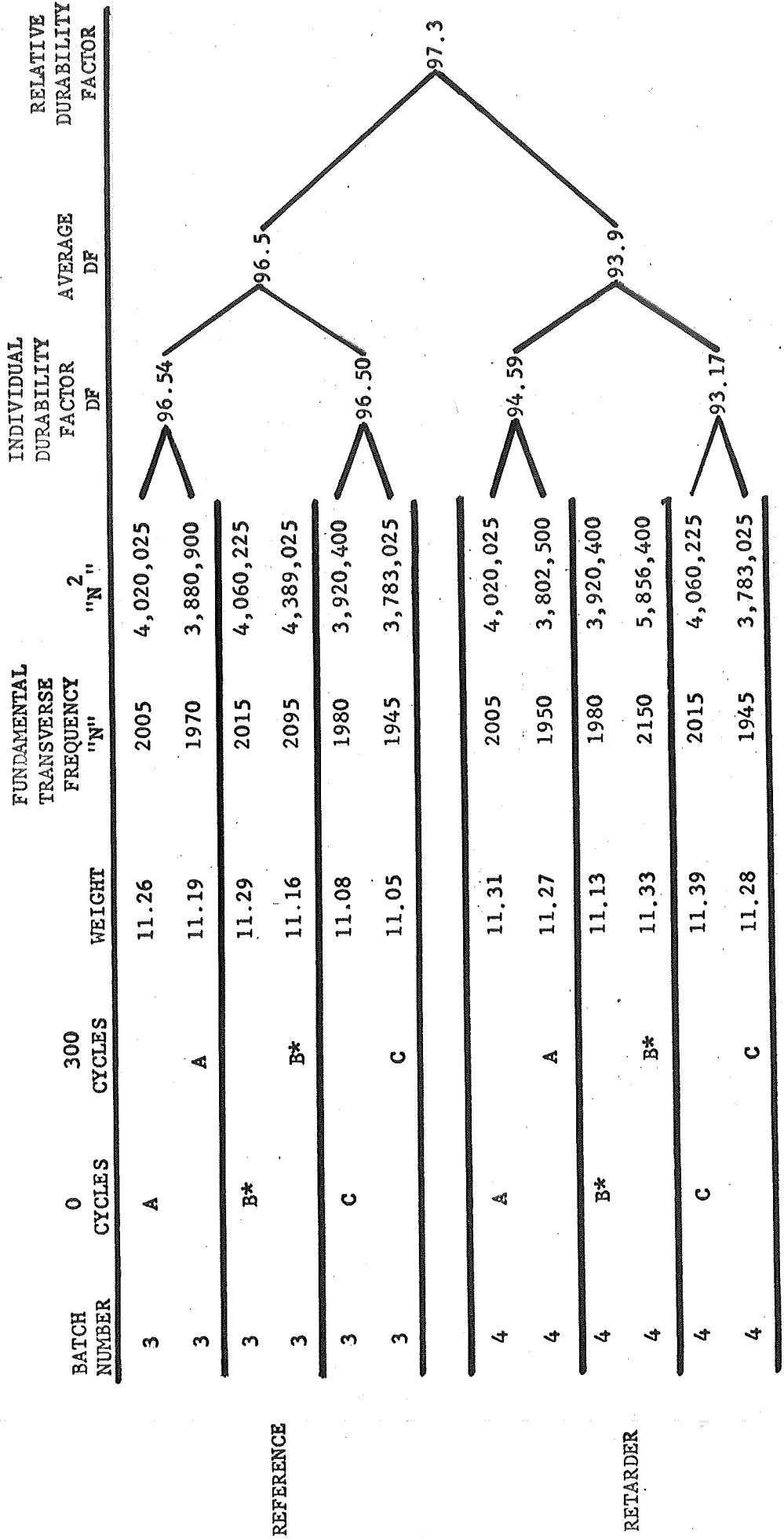


FIGURE 4C RELATIVE DURABILITY FACTOR - FREEZE THAW SPECIMENS
BATCHES 3 & 4

Sonic PSI

		At 0 Cycles	At 300 Cycles	Modulus of Rupture PSI	Compressive ** Strength PSI
1	A	6700	6600	800	6242
	B	6400	6300	1091	5621
	C*	7200	8600	873	6527
2					
	A*	7900	12600	1062	6606
	B	7700	7800	960	6467
3	C	7900	7600	880	7224
	A	10400	9800	1135	7685
4	B*	10600	11650	1236	7167
	C	9000	9250	1164	8055
	A	10500	9650	1164	8521
	B*	9850	17100	1033	8328
	C	10550	9550	1164	8545

*Continuously Moist Cured

**Average of both ends of beam broken in compression

FIGURE 4D STRENGTH TEST RESULTS FREEZE THAW SPECIMENS

APPENDIX

RETARDER TEST REPORT

"A"

DATE Feb. 16, 1970

Coarse Agg. Source Lebanon Crushed Stone			Cement Source. Northeast Cement Company					
Fine Agg. Source Lebanon Crushed Stone			Type 1					
Gradation % Passing			Retarder Source Grace Construction Products					
1"	100.0	3/8"	100.0	Trade Name Daratard H C				
3/4"	94.5	#4	100.0	Chemical Type Hydroxylated Carboxylic Acid				
3/8"	22.5	#16	67.5	Air Agent Source Grace Construction Products				
#4	1.7	#30	37.0	Trade Name Darex				
#8	0.9	#50	10.0	Class AA Mix Design				
Flats %	5.1	#100	2.5	Weights	Per Yard	Reference		
Fractures %	100.0	FM 2.92 Color	-1	#2 Stone	1770	98lbs 5oz		
Fine Agg. Sp. Gr.	2.74			Sand	1273	70lbs 12oz		
Coarse Agg. Sp. Gr.	2.71			Cement	611	33lbs 15oz		
Predominant Mineral Composition				Water	285	15.52lbs Avg		
Coarse Agg. Feldspathic Gneiss				Retarder	2.5oz/sk	26ml		
				A.E.A.	11-12g	10 $\frac{1}{2}$ g		
Fine Agg.	Quartz			Yield	101.1	100.7		
				Unit Weight		144.5lbs		

CONCRETE TESTS

Batch No.	Proctor Penetration		Air Content Percent	Temp. Degrees	Slump Inches	Compressive Strength		
	500 psi	4000 psi				3 Days	7 Days	28 Days
Retarder	Hrs.	Hrs.						
1	6.85	8.90	5 $\frac{1}{4}$	72	2 $\frac{3}{4}$	4041	4616	*4297
2	7.82	9.69	5 $\frac{1}{4}$	73	2 $\frac{1}{2}$	3855	4403	5535
3	8.41	10.62	5	74	2 $\frac{3}{4}$	3767	4244	5429
Average	7.69	9.74				3888	4421	5482
Reference								
4	4.74	6.71	5	74	3	2935	3519	4527
5	4.56	6.65	5	75	2 $\frac{1}{2}$	3280	3445	4439
6	4.58	6.61	6	74	2 $\frac{1}{2}$	2582	3210	4174
Average	4.63	6.66				2932	3391	4380

Water Content (Avg. % of reduction due to Retarder) 5.0

Comp. Strength (Avg. % of increase due to Retarder) 33 3 Days 30 7 Days 25 28 Days

Time of Setting (Avg. % of Deviation due to Retarder) 66 Initial 46 Final

Time of Setting (Avg. Increase in Hrs. due to Retarder) 3.06 Initial 3.08 Final

Notes: *Batch #1 28Day Break Not Included In Averages

RETARDER TEST REPORT

"B"

DATE Feb. 24, 1970

Coarse Agg. Source Carrara No. Clarendon	Cement Source Glens Falls Cement Company
Fine Agg. Source Carrara No. Clarendon	Type 1
Gradation % Passing	Retarder Source Grace Construction Products
1" 100.0 3/8" 100.0	Trade Name Daratard
3/4" 99.2 #4 99.0	Chemical Type Lignosulfonic Acid
3/8" 34.9 #16 75.0	Air Agent Source Grace Construction Products
#4 4.5 #30 44.0	Trade Name Darex
#8 1.9 #50 15.0	Class AA Mix Design
Flats % 5.4 #100 5.0	Weights Per Yard Reference Retarder
Fractures % 82.9 FM 2.69 Color -1	#2 Stone 1728 96lbs 0oz 96lbs 0oz
Fine Agg. Sp. Gr. 2.70	Sand 1196 66lbs 8oz 66lbs 8oz
Coarse Agg. Sp. Gr. 2.60	Cement 611 33lbs 15oz 33lbs 15oz
Predominant Mineral Composition	Water 285 16.75lbs Avg 15.17lbs Avg
Coarse Agg. Quartzite	Retarder 9oz/sk 92ml
Fine Agg. Quartz	A.E.A. 10ml 7-10ml
	Yield 100.6
	Unit Weight

CONCRETE TESTS

Batch No.	Proctor Penetration		Air Content Percent	Temp. Degrees	Slump Inches	Compressive Strength		
	500 psi	4000 psi				3 Days	7 Days	28 Days
Retarder	Hrs.	Hrs.						
1	8.85	12.50	6 $\frac{1}{2}$	71	1 $\frac{1}{2}$	4253	4297	4554
*2	---	---	8	70	3	2467	3289	3767
3	9.92	12.46	7	70	2 $\frac{1}{2}$	3387	4138	4793
Average	9.39	12.48				3820	4218	4674
Reference								
4	6.23	8.64	6	68	3	2953	3793	4403
5	5.97	8.79	5 $\frac{1}{2}$	70	3	3334	3599	4793
6	6.03	8.48	5 $\frac{1}{2}$	70	2 $\frac{1}{2}$	2785	3652	4359
Average	6.08	8.64				3024	3681	4518

Water Content (Avg. % of reduction due to Retarder)	9.4			
Comp. Strength (Avg. % of increase due to Retarder)	26	3 Days	15	7 Days 3 28 Days
Time of Setting (Avg. % of Deviation due to Retarder)	54	Initial	44	Final
Time of Setting (Avg. Increase in Hrs. due to Retarder)	3.31	Initial	3.84	Final

Notes: *Batch #2 Not Used In Averages

RETARDER TEST REPORT

"C"DATE March 3, 1970

Coarse Agg. Source Calkins Coventry			Cement Source. Northeast Cement Company					
Fine Agg. Source Calkins Coventry			Type 2					
Gradation % Passing			Retarder Source Sika Chemical Corporation					
1"	100.0	3/8"	100.0	Trade Name	Plastiment			
3/4"	94.0	#4	100.0	Chemical Type	Hydroxylated Carboxylic Acid			
3/8"	42.2	#16	72.0	Air Agent Source	Grace Construction Products			
#4	7.0	#30	46.0	Trade Name	Darex			
#8	2.8	#50	18.0	Class AA Mix Design				
Flats %	4.4	#100	6.0	Weights	Per Yard	Reference	Retarder	
Fractures %	86.7	FM 2.62 Color	1½	#2 Stone	1891	105lbs 1oz	105lbs 1oz	
Fine Agg. Sp. Gr.	2.76			Sand	1159	64lbs 6oz	64lbs 6oz	
Coarse Agg. Sp. Gr.	2.78			Cement	611	33lbs 15oz	33lbs 15oz	
Predominant Mineral Composition				Water	285	15.75lbs Avg	15.06lbs Avg	
Coarse Agg. Quartz and Labradorite				Retarder	2.5oz/sk		26ml	
Fine Agg. Quartz and Labradorite				A.E.A.		12-16ml	10-12ml	
				Yield	101.2	98.5	98.1	
				Unit Weight		147.9lbs	148.4lbs	

CONCRETE TESTS

Batch No.	Proctor Penetration		Air Content Percent	Temp. Degrees	Slump Inches	Compressive Strength		
	500 psi	4000 psi				3 Days	7 Days	28 Days
Retarder	Hrs.	Hrs.						
1	9.00	11.73	5	75	3	3218	4149	4757
2	8.95	12.15	4½	74	2½	3272	3581	*4183
3	9.00	12.00	5½	74	2½	2511	3431	5173
Average	8.98	11.96				3000	3720	4965
Reference								
4	6.00	8.90	5½	75	2½	2502	3749	4793
5	6.30	8.78	5	73	2½	2520	2829	4563
6	6.63	9.25	5	72	2½	2087	2989	4545
Average	6.31	8.98				2370	3189	4634

Water Content (Avg. % of reduction due to Retarder)	4.6						
Comp. Strength (Avg. % of increase due to Retarder)	27	3 Days	17	7 Days	7	28 Days	
Time of Setting (Avg. % of Deviation due to Retarder)	42	Initial	33	Final			
Time of Setting (Avg. Increase in Hrs. due to Retarder)	2.67	Initial	2.98	Final			

Notes: *Batch #2 28Day Break Not Included In Averages

RETARDER TEST REPORT

"D"DATE March 6, 1970

Coarse Agg. Source Demers Winooski			Cement Source Glens Falls Cement Co.				
Fine Agg. Source Griswold Williston			Type 1				
Gradation % Passing			Retarder Source Grace Construction Products				
1"	100.0	3/8"	100.0	Trade Name	Daratard		
3/4"	100.0	#4	100.0	Chemical Type	Lignosulfonic Acid		
3/8"	39.1	#16	64.0	Air Agent Source	Grace Construction Products		
#4	8.9	#30	47.0	Trade Name	Darex		
#8	2.2	#50	22.0	Class AA Mix Design			
Flats %	13.1	#100	5.0	Weights	Per Yard	Reference	Retarder
Fractures %	100.0	FM 2.79 Color	-1	#2 Stone	1811	100lbs 10oz	100lbs 10oz
Fine Agg. Sp. Gr.	2.68			Sand	1260	70lbs 0oz	70lbs 0oz
Coarse Agg. Sp. Gr.	2.79			Cement	611	33lbs 15oz	33lbs 15oz
Predominant Mineral Composition				Water	285	15.73lbs Avg	14.65lbs Avg
Coarse Agg.	Dolomite			Retarder	9oz/sk		92ml
				A.E.A.		10-15ml	4-8ml
Fine Agg.	Quartz			Yield	101.0	99.3	97.3
				Unit Weight		148.0	150.2

CONCRETE TESTS

Batch No.	Proctor Penetration		Air Content Percent	Temp. Degrees	Slump Inches	Compressive Strength		
	500 psi	4000 psi				3 Days	7 Days	28 Days
Retarder	Hrs.	Hrs.						
1	8.25	10.15	7 $\frac{1}{2}$	77	2 $\frac{3}{4}$	3440	4138	4227
2	7.33	9.85	4 $\frac{1}{2}$	75	1 $\frac{3}{4}$	4899	5217	6048
3	7.72	9.76	5	76	2 $\frac{1}{4}$	3749	4863	5394
Average	7.77	9.92				4029	4739	5223
Reference								
4	5.05	7.10	5	75	2 $\frac{1}{2}$	2502	4094	4200
5	5.29	7.32	5 $\frac{1}{2}$	74	2 $\frac{1}{4}$	3749	3988	4342
6	5.78	7.97	6	70	3	2935	3272	4147
Average	5.37	7.46				3062	3785	4230

Water Content (Avg. % of reduction due to Retarder) 6.9

Comp. Strength (Avg. % of increase due to Retarder) 32 3 Days 25 7 Days 24 28 Days

Time of Setting (Avg. % of Deviation due to Retarder) 45 Initial 33 Final

Time of Setting (Avg. Increase in Hrs. due to Retarder) 2.40 Initial 2.46 Final

Notes:

RETARDER TEST REPORT

"E 1"DATE March 13, 1970

Coarse Agg. Source Dailey So. Shaftsbury	Cement Source Glens Falls Cement Co.
Fine Agg. Source Dailey So. Shaftsbury	Type 1
Gradation % Passing	Retarder Source Master Builders Co.
1" 100.0	Trade Name MBHC
3/4" 99.4	Chemical Type Hydroxylated Carboxylic Acid
3/8" 35.9	Air Agent Source Master Builders Co.
#4 3.5	Trade Name IBVR
#8 2.3	Class AA Mix Design
Flats % 8.0	Weights Per Yard
Fractures % 70.0	#2 Stone 1800 100lbs 0oz 100lbs 0oz
Fine Agg. Sp. Gr. 2.76	Sand 1223 67lbs 15oz 67lbs 15oz
Coarse Agg. Sp. Gr. 2.71	Cement 611 33lbs 15oz 33lbs 15oz
Predominant Mineral Composition	Water 285 16.40lbs Avg 16.00lbs Avg
Coarse Agg. Quartz and Calcite	Retarder 3oz/sk 31ml
A.E.A.	
Fine Agg. Quartz	Yield 100.2 100.2 100.0
	Unit Weight 145.25 145.50

CONCRETE TESTS

Batch No.	Proctor Penetration		Air Content Percent	Temp. Degrees	Slump Inches	Compressive Strength		
	500 psi	4000 psi				3 Days	7 Days	28 Days
Retarder	Hrs.	Hrs.						
1	9.30	11.88	4 $\frac{3}{4}$	70	2	4740	4757	5474
2	8.77	10.88	5 $\frac{1}{2}$	70	2	4439	4722	5801
3	8.70	10.88	6	70	2 $\frac{1}{2}$	3961	4704	5341
Average	8.92	11.21				4380	4728	5539
Reference								
4	5.40	7.48	4 $\frac{1}{2}$	70	1 $\frac{3}{4}$	4103	4881	4032
5	5.50	7.50	4 $\frac{1}{2}$	72	2 $\frac{1}{4}$	3837	3926	5235
6	6.10	8.27	5 $\frac{1}{2}$	72	2 $\frac{1}{2}$	3139	3564	4386
Average	5.67	7.75				3693	4124	4551

Water Content (Avg. % of reduction due to Retarder) 2.4

Comp. Strength (Avg. % of increase due to Retarder) 19 3 Days 15 7 Days 22 28 Days

Time of Setting (Avg. % of Deviation due to Retarder) 57 Initial 45 Final

Time of Setting (Avg. Increase in Hrs. due to Retarder) 3.25 Initial 3.46 Final

Notes: Reference Mixes Displayed False Set During 3min. Waiting Period.

RETARDER TEST REPORT

"E 2"DATE March 17, 1970

Coarse Agg. Source		Shaftsbury			
Fine Agg. Source		Cement Source			
Gradation % Passing		Type			
1"	3/8"	Retarder Source Master Builders Co.			
3/4"	#4	Trade Name Pozzolith 100XR			
3/8"	#16	Chemical Type Carbohydrate			
#4	#30	Air Agent Source			
#8	#50	Trade Name			
Flats %	#100	Class AA Mix Design			
Fractures %	FM	Color	Weights	Per Yard	Reference
Fine Agg. Sp. Gr.			#2 Stone		Retarder
Coarse Agg. Sp. Gr.			Sand		100 lbs 0 oz.
Predominant Mineral Composition			Cement		67 lbs 15 oz.
Coarse Agg.			Water		33 lbs 15 oz.
			Retarder		16.44 lbs Avg.
			A.E.A.		31ml
Fine Agg.			Yield		12-18ml
			Unit Weight		100.2
					145.25 lbs

CONCRETE TESTS

Batch No.	Proctor Penetration		Air Content Percent	Temp. Degrees	Slump Inches	Compressive Strength		
	500 psi	4000 psi				3 Days	7 Days	28 Days
Retarder	Hrs.	Hrs.						
1	9.60	11.88	4	74	2 $\frac{1}{2}$	4191	5305	5474
2	10.67	12.81	4 $\frac{3}{4}$	74	2 $\frac{1}{2}$	4014	4536	5456
3	10.50	12.61	5	74	2 $\frac{1}{2}$	3643	4439	5252
Average	10.26	12.43				3949	4760	5394
Reference								
4								
5								
6								
Average								

Water Content (Avg. % of reduction due to Retarder) "No Reduction" Increased 0.2%

Comp. Strength (Avg. % of increase due to Retarder) 7 3 Days 15 7 Days 19 28 Days

Time of Setting (Avg. % of Deviation due to Retarder) 81 Initial 60 Final

Time of Setting (Avg. Increase in Hrs. due to Retarder) 4.59 Initial 4.68 Final

Notes: Reference Batches Listed on Page 1. Tests Performed On March 13, 1970

RETARDER TEST REPORT

"F"DATE March 20, 1970

Coarse Agg. Source Kelley Websterville	Cement Source. Universal Atlas Cement
Fine Agg. Source Kelley Websterville	Type 1
Gradation % Passing	Retarder Source Grace Construction Products
1" 100.0 3/8" 100.0	Trade Name Daratard
3/4" 99.7 #4 100.0	Chemical Type Lignosulfonic Acid
3/8" 45.9 #16 73.5	Air Agent Source Grace Construction Products
#4 8.6 #30 50.0	Trade Name Darex
#8 2.5 #50 23.0	Class AA Mix Design
Flats % 6.9 #100 5.0	Weights Per Yard Reference Retarder
Fractures % 100.0 FM 2.59 Color -1	#2 Stone 1654 91lbs 14oz 91lbs 14oz
Fine Agg. Sp. Gr. 2.72	Sand 1253 69lbs 10oz 69lbs 10oz
Coarse Agg. Sp. Gr. 2.60	Cement 611 33lbs 15oz 33lbs 15oz
Predominant Mineral Composition	Water 285 16.15lbs Avg 14.81lbs Avg
Coarse Agg. Granodiorite	Retarder 9oz/sk 92ml
	A.E.A. 10-16ml 6-8ml
Fine Agg. Quartz	Yield 99.7 98.2 98.0
	Unit Weight 143.75 143.25

CONCRETE TESTS

Batch No.	Proctor Penetration		Air Content Percent	Temp. Degrees	Slump Inches	Compressive Strength		
	500 psi	4000 psi				3 Days	7 Days	28 Days
Retarder	Hrs.	Hrs.						
1	7.90	9.33	7 1/2	70	2 1/2	2759	3210	3599
2	8.50	10.10	6 1/4	70	2 1/2	3395	3422	3979
3	8.40	9.73	6	72	2	3245	3723	4695
Average	8.27	9.72				3133	3452	4091
Reference								
4	4.35	5.65	4 1/2	72	2 1/4	3192	3820	3882
5	4.25	5.70	5	71	2	3148	2918	3882
6	4.35	5.78	5 3/4	69	2 1/4	2440	2661	3564
Average	4.32	5.71				2927	3133	3776

Water Content (Avg. % of reduction due to Retarder) 7.1

Comp. Strength (Avg. % of increase due to Retarder) 7 3 Days 10 7 Days 8 28 Days

Time of Setting (Avg. % of Deviation due to Retarder) 91.4 Initial 70.2 Final

Time of Setting (Avg. Increase in Hrs. due to Retarder) 3.95 Initial 4.01 Final

Notes: